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PRILOGA 1C

NASLOVNA STRAN NAČRTA

		Psihiatrična bolnišnica Begunje
PODATKI O GRADNJI		
naziv gradnje		**STAVBA ZA ZDRAVSTVENO OSKRBO DNEVNA AMBULANTA**
kratek opis gradnje		Rekonstrukcija in sprememba namembnosti obstoječe stavbe (mizarske delavnice) v dnevno ambulanto v sklopu PBB.
VRSTE GRADNJE *označiti vse ustrezne vrste gradnje*	☐	**NOVOGRADNJA - NOVOZGRAJEN OBJEKT**
	☐	**NOVOGRADNJA - PRIZIDAVA**
	☑	**REKONSTRUKCIJA**
	☑	**SPREMEMBA NAMEMBNOSTI**
	☐	**ODSTRANITEV CELOTNEGA OBJEKTA**
	☐	**LEGALIZACIJA**
	☐	**MANJŠA REKONSTRUKCIJA**
PODATKI O PROJEKTNI DOKUMENTACIJI		
vrsta dokumentacije		**PZI**
številka projekta		**PBB-01-24-DB**
PODATKI O NAČRTU		
strokovno področje načrta		**načrt s področja gradbeništva**
naziv načrta		**GRADBENA KONSTRUKCIJA**
številka načrta		**2025-7**
datum izdelave		**april 2025**
datum spremembe		
PODATKI O PROJEKTANTU NAČRTA		
projektant načrta (naziv družbe)		**STATIKA FONDA, Diana Fonda s.p.**
naslov		**Maistrova 22, 4240 Radovljica**
odgovorna oseba projektanta načrta		**Diana FONDA, univ.dipl.inž.gradb.**
podpis odgovorne osebe projektanta načrta		STATIKA FONDA NAČRTOVANJE GRADBENIH KONSTRUKCIJ Diana Fonda s.p., Maistrova 22, Radovljica
PODATKI O IZDELOVALCU NAČRTA		
ime in priimek pooblaščenega arhitekta, pooblaščenega inženirja		**Diana FONDA, univ.dipl.inž.gradb.**
identifikacijska številka		**IZS G-1419**
podpis pooblaščenega arhitekta, pooblaščenega inženirja		DIANA FONDA univ. dipl. inž. grad. IZS G-1419

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PRILOGA 2C

IZJAVA PROJEKTANTA NAČRTA IN POOBLAŠČENEGA STOKOVNJAKA, KI JE IZDELAL NAČRT V PZI IN PID

	Psihiatrična bolnišnica Begunje
PROJEKTANT NAČRTA	
projektant načrta (naziv družbe)	**STATIKA FONDA, Diana Fonda s.p.**
naslov	**Maistrova 22, 4240 Radovljica**
odgovorna oseba projektanta načrta	**Diana FONDA, univ.dipl.inž.gradb.**
IN POOBLAŠČENI STROKOVNJAK, KI JE IZDELAL NAČRT	
pooblaščeni strokovnjak	**Diana FONDA, univ.dipl.inž.gradb.**

IZJAVLJAVA:

da načrt

vrsta dokumentacije	**PZI**
strokovno področje načrta	**načrt s področja gradbeništva**
naziv načrta	**GRADBENA KONSTRUKCIJA**
številka načrta	**2025-7**
datum izdelave	**april 2025**

upošteva relevantne predpise in druge normativne dokumente ter da so upoštevane ustrezne bistvene in druge zahteve.

pooblaščeni strokovnjak	**Diana FONDA, univ.dipl.inž.gradb.**
identifikacijska številka	**IZS G-1419**
podpis pooblaščenega strokovnjaka	DIANA FONDA univ. dipl. inž. grad. IZS G-1419
odgovorna oseba projektanta načrta	**Diana FONDA, univ.dipl.inž.gradb.**
podpis odgovorne osebe projektanta načrta	STATIKA FONDA NAČRTOVANJE GRADBENIH KONSTRUKCIJ Diana Fonda s.p., Maistrova 22, Radovljica

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VSEBINA NAČRTA

1C Naslovna stran načrta

2C Izjava projektanta načrta in pooblaščenega strokovnjaka, ki je izdelal načrt

Tehnično poročilo

Tehnični opis
Statični račun

Tehnični prikazi

1. Pozicijski načrt pritličja
2. Pozicijski načrt mansarde
3. Pozicijski načrt ostrešja
4. Armaturni načrt temeljev
5. Armaturni načrt sten
6. Armaturni načrt dvigalnega jaška
7. Armaturni načrt plošče nad pritličjem
8. Rekapitulacija armature
9. Sidranje slemenske lege nadstreška
10. Sidranje kapne lege nadstreška

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TEHNIČNO POROČILO

TEHNIČNI OPIS

Splošno:

Načrt gradbenih konstrukcij obravnava posege v nosilno konstrukcijo v sklopu projekta »Stavba za zdravstveno oskrbo - dnevna ambulanta«, investitorja je Psihiatrična bolnišnica Begunje. Predmet projekta je rekonstrukcija in sprememba namembnosti obstoječe stavbe (mizarske delavnice) v dnevno ambulanto v sklopu PBB.

Predvideni materiali:

armirani beton C25/30 , XC1 in XC2
armatura S500
les C24
opeka MO10
cementna malta MM5

Uporabljeni predpisi:

- EUROCODE 1 Osnove projektiranja in vplivi na konstrukcije
- EUROCODE 2 Projektiranje betonskih konstrukcij
- EUROCODE 3 Projektiranje jeklenih konstrukcij
- EUROCODE 5 Projektiranje lesenih konstrukcij
- EUROCODE 6 Projektiranje zidanih konstrukcij
- EUROCODE 7 Geotehnično projektiranje
- EUROCODE 8 Projektiranje konstrukcij na potresnih področjih

Privzete obtežbe:

sneg:	nadmorska višina	nm=	570	m
cona A3	karakt. vrednost obtežbe snega:	s_k =	3,12	kN/m^2
	naklon:	α =	40	°
	oblikovni koeficient:	μ_1 =	0,53	

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	koeficient izpostavljenosti:	C_e =	1,00	
	termični koeficient:	C_t =	1,00	
	obtežba snega:	$s = s_k * c_e * c_t * \mu_1$ =	**1,66**	**kN/m²**
veter	referenčna hitrost vetra:	v_{ref} =	25,00	m/s
cona 2	gostota zraka:	ρ =	1,25	kg/m³
	karakteristična btežba vetra:	$q_{ref} = \rho * v_{ref}^2 / 2$ =	0,25	kN/m²
	koeficient izpostavljenosti:	ce =	1,65	
		$we = q_{ref} * ce$ =	**0,77**	**kN/m²**

koristna obtežba

Kategorija B - pisarne	qk =	**3,00**	**kN/m²**
redukcijski faktor	$\alpha_A = 5*\Psi_0/7+A_0/A$ =	**1,00**	
Ψ faktor za stavbe	Ψ_0 =	**0,70**	
računska obtežba	$q = \alpha_A * q_k$ =	**2,25**	**kN/m²**

Seizmika

Lokacija spada v cono, kjer je predpisan pospešek 0,225 g po karti iz leta 2021 »Potresna nevarnost Slovenije – projektni pospeški tal« s povratno dobo 475 let in tipom tal C (tla po EC8).

Opis nosilne konstrukcije

TEMELJENJE

Obstoječe temeljenje JV dela objekta se predvidoma ohranja. Utrdi naj se jih z injektiranjem ali pa po potrebi s podbetoniranjem in obbetonirarjem.

SZ del objekta je v celoti nov, predvideni so armirano betonski pasovni temelji.

PRITLIČJE

Vertikalna nosilna konstrukcija JV dela, ki se ohranja, je zgrajena kot kombinacija kamnitih sten različnih debelin. Poglobijo in očistijo naj se eventuelne razpoke, ki se sanirajo z injektiranjem in ometavanjem razpok. Uporabi naj se injekcijska masa na hidravlični apneni NHL bazi (kot npr. injekcijska malta Rofix Sismadur Iniezione 15 ali pa Mapei Mape Antique-I). V primeru, da se ob gradnji izkaže slaba kvaliteta kamnitih zidov, naj se le ti sistematično injektirajo.

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SZ del bo zidan z opečnim modularnim blokom debeline 30 cm v sistemu povezanega zidovja.

Medetažna konstrukcija bo v celoti nova in sicer armirano betonska plošča debeline 22 cm, nad odprtinami pa so nove armirano betonske preklade širine 30 cm.

Predvidena je restavratorska obnova nadstreška, ki se bo sidral v novo AB konstrukcijo z jeklenim T profilom, vstavljenim v sredinski utor.

MANSARDA

V mansardi se izvede nova nosilna vertikalna konstrukcija iz sten debeline 30 cm v sistemu povezanega zidovja z ustreznimi vertikalnimi in horizontalnimi armirano betonskimi vezmi.

OSTREŠJE

Novo ostrešje je sistem dvokapnice, špiravci nalegajo na kapni legi, med seboj so povezani s kleščami po 2x 5/20 cm. Horizontalno zavetrovanje se lahko izvede s perforiranimi trakovi, ki naj se v diagonalnih križih pritrjujejo na vrhu špiravcev ali pa s tesno privijačenimi OSB ploščami. Krajni špiravci naj se vijačijo v zidno AB vez čelnih fasadnih sten, enako velja za špiravce v linijah vmesnih prečnih sten mansarde.

Požarna varnost

Načrt gradbene konstrukcije je izdelan z upoštevanjem Načrta požarne varnosti št. 25 02 23. ki ga je maja 2025 izdelalo podjetje I.S.P., d.o.o. . Zahtevana požarna odpornost nosilne konstrukcije je R60.
V skladu s Tehnično smernico TSG-1-001:2019 Požarna varnost v stavbah – poglavje Nosilnost konstrukcije ter širjenje požara po stavbah (točka 2.1(3) je dokaz o požarni odpornosti nosilne konstrukcije stavbe klasifikacija proizvodov po standardu SIST EN 13501-2 Požarna klasifikacija gradbenih proizvodov in elementov stavb glede odziva na ogenj. Nosilni materiali, predvideni v načrtovanem objektu, to je beton in opeka, spadajo v razred A1, les pa v razred D.
Glede na podatek iz nemških predpisov DIN 4102-4, da za požarno odpornost R90 zadostuje minimalna debelina armirano betonske plošče že 10 cm, mi pa imamo 22 cm, smatram, da poseben račun nosilnosti oziroma uporabnosti ni potreben. Minimalna zaščitna plast betona preko armature je 25 mm, kar pomeni, da ob standardnemu požaru po 30 min doseže beton na površini 760°C, v globini 25 mm pa le 280°C. Ta temperatura pa še ne vpliva na mehanske lastnosti jekla.
Podobno so glede na požar predimenzionirane nosilne opečne stene, ki so debeline 30 cm.

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Projektant:

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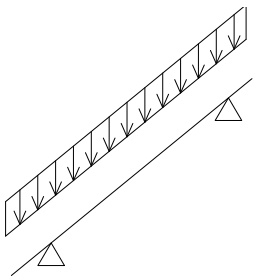
STATIČNI RAČUN

OSTREŠJE

LES C24

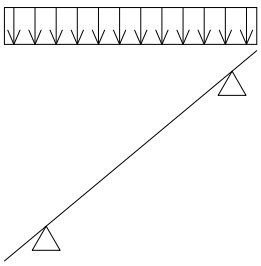
$f_{m,d}$ =	1,477	kN/cm^2	$E_{0,05}$ =	740	kN/cm^2
$f_{t,0,d}$ =	0,862	kN/cm^2	$E_{0,mean}$ =	1100	kN/cm^2
$f_{t,90,d}$ =	0,025	kN/cm^2			
$f_{c,0,d}$ =	1,292	kN/cm^2	α =	40	°
$f_{c,90,d}$ =	0,326	kN/cm^2	**A** ~	570	m
$f_{v,d}$ =	0,154	kN/cm^2			

OBTEŽBA:



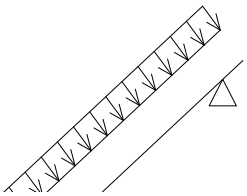
lastna + stalna obtežba

kritina	0,60	kN/m^2
dvojno letvanje	0,06	kN/m^2
toplotna izolacija	0,25	kN/m^2
škarniki	0,15	kN/m^2
stropna obloga	0,25	kN/m^2
g` =	1,31	kN/m^2
g = g` / cos α =	**1,71**	kN/m^2



sneg:

cona A 3 f =	1,94	
s_k = f * [1+(A / 728)^2] =	3,12	kN/m^2
μ_1 = 0,8 * (60 - a) / 30 =	0,53	
c_e =	1,0	
c_z =	1,0	
s = μ_k * C_e * C_1 * s_k =	**1,66**	**kN/m^2**



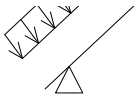
veter:

cona:	2	v_{ref} =	25	m/s

$w_e = q_{ref} * c_e(z_e) * c_{pe}$

$w_i = q_{ref} * c_i(z_i) * c_{pi}$

$w = w_e + w_i$



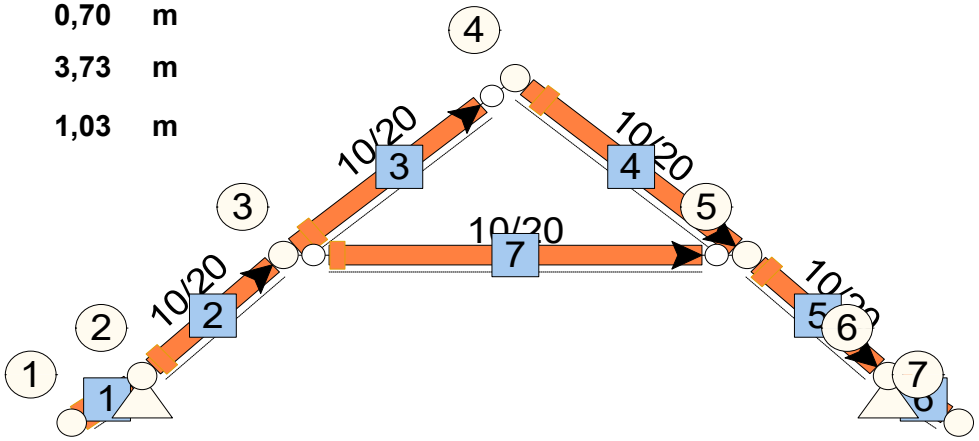
$q_{ref} = \rho/2 * v_{ref}^2 =$	0,39	kN/m^2
c_e (kat. terena III, h = 8m) =	1,65	
c_i =	1,65	
c_{pe} (dvokapnica 45°) =	-0,30	0,70
	srk	pritisk
c_{pi} =	-0,25	0
$w^+ = q_{ref} * c_e(z_e) * (c_{pe}+c_{pi}) / \cos^2 \alpha$	**0,77**	**kN/m^2**
$w^- = q_{ref} * c_e(z_e) * (c_{pe}+c_{pi}) / \cos^2 \alpha$	**-0,60**	**kN/m^2**

KOMBINACIJE OBTEŽB:

MSN:	1.	1.35 g + 1.5 s + 0.9 w+	q1 =	5,50	kN/m^2
	2.	1.35 g + 0.75 s + 1.5 w+	q2 =	2,65	kN/m^2
	3.	1.00 g + 1.5w-	q3 =	0,80	kN/m^2
MSU:	4.	g + s + 0,6 w	q4 =	3,84	kN/m^2
	5.	g + 0.5 s + w	q5 =	3,31	kN/m^2

POZ 301 špirovci

L_k =	**0,70**	**m**
L_p =	**3,73**	**m**
e =	**1,03**	**m**



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Vpliv	stalna obtežba	$g_1 = g * e =$	1,76	kN/m
	sneg	$s_1 = s * e =$	1,71	kN/m
	veter	$w_1 = w * e =$	0,79	kN/m

Učinek vpliva izračun s programom FRAME 2D - priloga 1

konzola (oslabljeni prerez)	$M_{sd} =$	1,47	kNm
polje	$M_{sd} =$	5,25	kNm

Dimenzioniranje:

sidranje špirovcev direktno skozi kapno lego v AB konstrukcijo

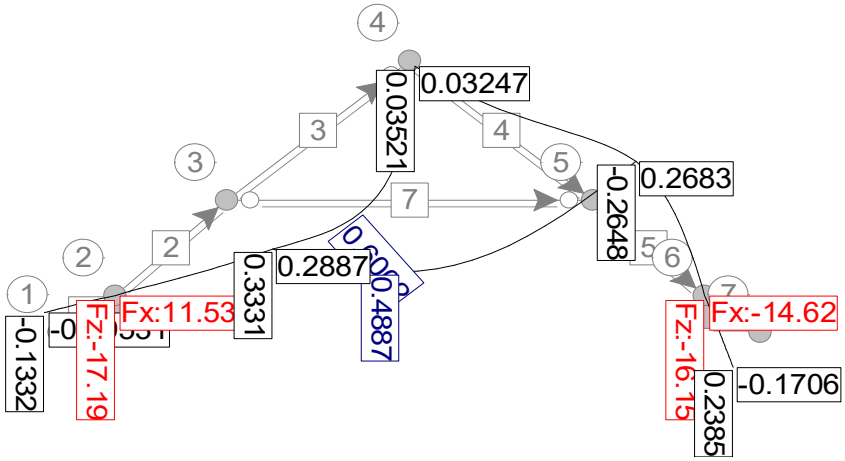
b =	**10**	**cm**	$W_p =$	667	cm^3
h =	**20**	**cm**	$I_p =$	6667	cm^4
zasek =	**3,0**	**cm**	$W_k =$	482	cm^3
			$I_k =$	4094	cm^4

kontrola napetosti - iteracija:

polje	$M_{sd}/W*f_{m,d} + N_{sd}/A*f_{c,0,d}*kc =$	**0,53**	**≤** 1
konzola	$M_{sd}/W*f_{m,d} + N_{sd}/A*f_{c,0,d}*kc =$	**0,21**	**≤** 1

Kontrola povesa:

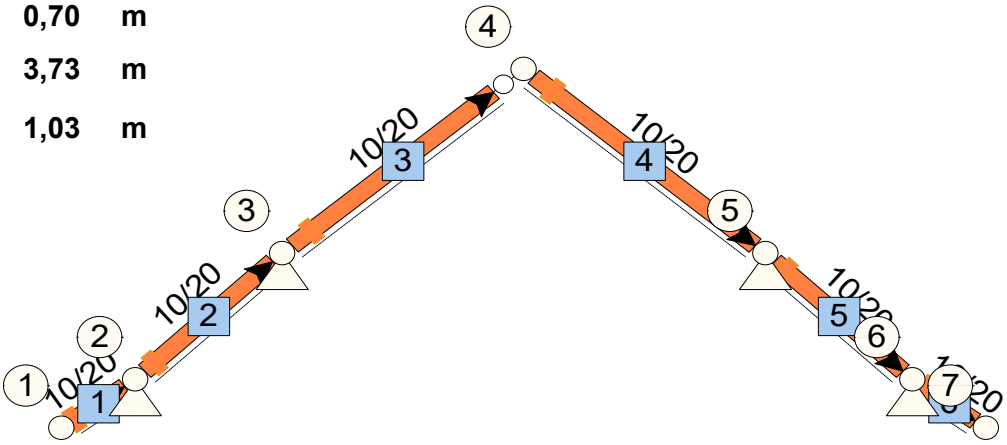
	$f_{max} =$	0,3	cm < f_{dop}
$f_{dop} = L_1 / 150 =$		1,5	cm



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POZ 301a špirovci

L_k = **0,70** **m**
L_p = **3,73** **m**
e = **1,03** **m**



Vpliv	stalna obtežba	$g_1 = g * e$ =	1,76	kN/m
	sneg	$s_1 = s * e$ =	1,71	kN/m
	veter	$w_1 = w * e$ =	0,79	kN/m
	klimat	G =	1,00	kN/m

Učinek vpliva izračun s programom FRAME 2D - priloga 2

konzola (oslabljeni prerez)	M_{sd} =	1,49	kNm
polje	M_{sd} =	3,03	kNm

Dimenzioniranje:

sidranje špirovcev direktno skozi kapno lego v AB konstrukcijo

b =	**10**	**cm**	W_p =	667	cm^3
h =	**20**	**cm**	I_p =	6667	cm^4
zasek =	**3,0**	**cm**	W_k =	482	cm^3
			I_k =	4094	cm^4

kontrola napetosti - iteracija:

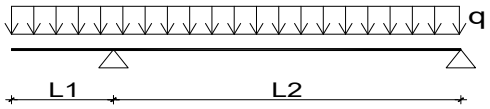
polje	$M_{sd}/W*f_{m,d} + N_{sd}/A*f_{c,0,d}*kc$ =	**0,31**	**≤** 1
konzola	$M_{sd}/W*f_{m,d} + N_{sd}/A*f_{c,0,d}*kc$ =	**0,21**	**≤** 1

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POZ 301b vmesna lega

Računski model

L1 =	**0,00**	**m**	
L2 =	**3,09**	**m**	
b =	**2,75**	**m**	(vplivno polje)



Vpliv:

$q_{MSN} = q_{1,2,3} * b =$ 15,12 kN/m

$q_{MSU} = q_{4,5} * b =$ 10,55 kN/m

Učinek vpliva

polje - merodajno $M_{sd} = q_{MSN} * L_2^2 / 8 - 0{,}45 * q_{MSN} * L_1^2 / 2 =$ 18,04 kNm

Dimenzioniranje:

MIN. PRESEK:

b =	**20**	**cm**	$W_{dej} =$	1333	cm^3
h =	**20**	**cm**	$I_{dej} =$	13333	cm^4

MEJNO STANJE NOSILNOSTI

kontrola napetosti - iteracija:

polje $M_{sd}/W*f_{m,d} + N_{sd}/A*f_{c,0,d}*kc =$ **0,92** **≤ 1**

MEJNO STANJE UPORABNOSTI

kontrola povesa:

polje MSU $M_{MSU} = q_{MSU} * L_2^2 / 12 - 0{,}45 * q_{MSU} * L_1^2 / 2 =$ 8,39 kNm

v polju: $f_{max} = M_{MSU} * L_2^2 / 9{,}6 * E * I =$ 0,6 cm < f_{dop}

$f_{dop} = L_2 / 250 =$ 1,2 cm

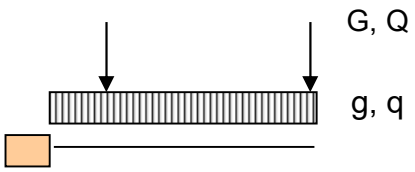
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POZ 302 Kapna lega

Dimenzioniranje:

minimalne dimenzije:	**b**	**=**	**20**	**cm**	W_{dej} =	1613	cm^3
	h	**=**	**22**	**cm**	I_{dej} =	17747	cm^4

POZ 310 LEGE NADSTREŠKA



L = 1,30 m
L_1 = 0,30 m

Vpliv

STALNA OBTEŽBA				
lastna teža		=	0,50	kN/m
stalna kontinuirna skupaj	g	=	0,50	kN/m
od strehe			1,45	kN
stalna koncentrirana skupaj	G	=	1,45	kN
SPREMENLJIVA OBTEŽBA				
od strehe			2,06	kN
koristna koncentrirana skupaj	Q	=	2,06	kN

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Učinek vpliva

maksimalni moment - absolutno:

M_{sd} = 0,5*(1,35 * g + 1,50 * q) * L^2 + (1,35*G + 1,50*Q)*(L+L_1) = 8,63 kNm

863 kNcm

Dimenzioniranje : konstrukcijsko jeklo S235

meja plastičnosti	fy =	23,50	kN/cm^2
natezna trdnost	fu =	36,00	kN/cm^2
E = 210000000 KN/m2	E =	21000	kNcm

γ_{M0} = 1,10

potrebni odpornostni moment: $W_{pot} = M_{max} * \gamma_{M0} / fy$ = 40,38 cm^3

pločevina višina: 200 mm Wdej = 80 cm^3

širina: 12 mm Idej = 800 cm^4

kontrola napetosti - iteracija: $M_{sd}/W*f_y + N_{sd}/A*f_y*kc$ = **0,46** **≤ 1**

Kontrola povesa:

dopustni poves: fdop = L / 250 = 0,5 cm

dejanski poves: fdej = (G * L^3 / 3 + (g + q) * L^4 / 8) / (E*I) = 0,2 cm

Sidranje

projektni moment:

M_{sd} = 8,6 kNm

razmak: a = 20,0 cm

število nateznih sider:: n = 2,0

natezna sila v sidru N_{Ed1} = Msd / (a * n) = 21,6 kN

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vijaki trdnostnega razreda 5.6

natezna nosilnost:

meja plastičnosti	f_{yb} =	40,00	kN/cm^2	
natezna trdnost	f_{ub} =	50,00	kN/cm^2	
	E =	21000	kN/cm^2	

γ_{M2} = 1,50

sidrna palica **2 kom. M 16** A = 4,02 cm^2

sila v 1 vijaku $N_{Ed1} = N_{Ed} / 2$ = 10,78 kN

karakteristična odpornost sidra: $N_{Rk,s}$ = 125,60 kN

projektna natezna nosilnost vijaka $F_{t,Rd} = N_{Rk,s} / \gamma_{M2}$ = 83,73 kN > N_{Ed1}

projektna natezna nosilnost vijaka $F_{t,Rd} = 0,9 * A * f_{ub} / \gamma_{M2}$ = 120,58 kN > N_{Ed1}

odpornost proti preboju pločevine

$B_{p,Rd} = 0,6 * \pi * f_u * d_m * t_p / \gamma_{M2}$ = 113,04 kN > > N_{Ed1}

debelina pločevine :	t_p =	1,20	cm	
debelina pločevine :	e =	1,20	cm	
premer luknje	s =	1,80	cm	
	$d_m = (e + s) / 2$ =	1,50	cm	

Potrebna dolžina sider v betonu

projektni moment:	M_{sd} =	863	kNcm	
razmak:	a =	20,0	cm	
število nateznih sider::	n =	2,0		
natezna sila v sidru	$F_{t,Ed}$ = Msd / (a * n) =	21,6	kN	
premer sidra	d =	1,6	cm	
ploščina preseka sidra	A_s =	2,0	cm^2	

projektna napetost v sidru: $\sigma_{sd} = F_{t,Ed} / A_s$ = 10,73 kN/cm^2

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projektna vrednost natezne trdnosti za C 25/30	f_{ctd} =	0,26	kN/cm^2	
koeficient, odvisen od pogojev sidranja	μ_1 =	1		
koeficient, odvisen od premera palice	μ_2 =	1		
projektna vrednost mejne napetosti sidra	$f_{bd} = 2,25 * \mu_1 * \mu_2 * f_{ctd}$ =	0,59	kN/cm^2	
osnovna sidrna dolžina	$l_{bd} = 0,25 * d * \sigma_{sd} / f_{ctd}$ =	7,34	cm	
minimalna sidrna dolžina	$l_{b,min} = 10 * d$ =	16,00	cm <	20 cm

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POZ 200 MANSARDA

POZ 201 KRIŽEM ARMIRANA PLOŠČA NAD DVIGALOM

Vpliv

STALNA OBTEŽBA

lastna teža plošče	b * h * 25,00		3,75	kN/m^2
tlak, strop			1,00	kN/m^2
stalni vpliv skupaj		**g =**	**4,75**	**kN/m^2**

SPREMENLJIVA OBTEŽBA

kategorija uporabe: H	površina A < 20,0	ψ_0 =	0,70	
koristna obtežba		q_k =	1,00	kN/m^2
koristna obtežba skupaj		**q =** α_A * q_k =	**1,00**	**kN/m^2**
redukcijski faktor:		α_A = (5/7) *ψ_0 + A_0 / A =	1,00	

koncentrirana od montažne kljuke **Q** = **10,00** **kN**

Učinek vpliva izračun s programom PLATE

maksimalni moment - polje M_{sd} = 7,85 kNm

Dimenzioniranje: izvedeno s programom PLATE

lastnosti materiala:

beton C25/30

f_{ck} = 2,50 kNcm^2

αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2

τ_{Rd} = 0,030 kN/cm^2

geometrijski podatki:

debelina plošče:

h = 15 cm

d = h - a = 13 cm

a = 2 cm

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armatura S500

f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 =43,48 kN/cm^2

mejno stanje uporabnosti izračun za maksimalno obremenitev v polju

RAZPOKE		$K_d = M_{sd} / \alpha f_{cd} * b * d^2$ =	0,033	$\Rightarrow$ kx =	0,091
		$x = k_x * h$ =	1,37	cm	
		$z = h - a - x/3$ =	12,55	cm	
		$N_{sd} = M_{sd} / z$ =	62,57	kN	
napetost jekla		N_{sd} / A_s =	18,68	kN/cm^2	
			187	Mpa	
načrtovana armatura:	**Q 335**	A_s =	3,35	cm^2	
		premer palic:	8	mm	
		razmak palic:	15	cm	

Armatura ustreza pogoju za omejitev širine razpok iz tabel 7.2N in 7.3N iz SIST EN 1992-1-1 za razpoko širine 0,3 mm. Dovoljena širina je 0,4 mm.

POVESI

merodajni razpon plošče:		L_p =	180	cm
dopusten poves:	f_{dop} = L_p / 250 =		0,72	cm
računski poves		f_{max} =	0,01	cm

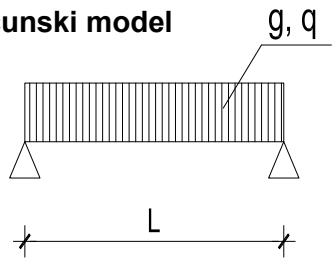
dejanski poves - upoštevano lezenje

$f_{eff} = f_{max} * (E_{cm} / E_{c,eff})$ =	0,04	cm < f_{dop}
E_{cm} =	31	Gpa
$E_{c,eff} = E_{cm} / (1 + \varphi(oo, t_0))$ =	8	Gpa
$\varphi(oo, t_{0)}$ =	3	(za beton star 30 dni)

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POZ 210 PROSTOLEŽEČ NOSILEC - preklada

Računski model



svetli razpon: **L_s = 2,20 m**

računski razpon: $L = L_s * 1,05$ = 2,31 m

Vpliv

STALNA OBTEŽBA

lastna teža	b * h * 25,00	=	1,50	kN/m
od strehe		=	1,78	kN/m
od stene		=	9,90	kN/m
stalna skupaj	g	=	13,18	kN/m

SPREMENLJIVA OBTEŽBA

sneg, veter		=	2,21	kN/m
koristna skupaj	q	=	2,21	kN/m

Učinek vpliva

$M_{sd} = 0,125 * (1,35 * g + 1,50 * q) * L^2$ = 14,07 kNm
1407 kNcm

max prečna sila
$V_d = 0,500 * (1,35 * g + 1,50 * q) * L$ = 24,37 kN

Dimenzioniranje:

lastnosti materiala:

beton C25/30

f_{ck} = 2,50 kNcm^2

αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2

τ_{Rd} = 0,030 kN/cm^2

minimalne dimenzije

b = 30 cm

h = 20 cm

d = 17 cm

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armatura S500

f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 = 43,48 kN/cm^2

pozitivna armatura

$K_d = M_{sd} / \alpha\, f_{cd} * b * d^2$ = 0,114 $\Rightarrow$ ks = 1,085

$A_s = k_s * M_{sd} / d * f_{yd}$ = 2,07 cm^2

	3	**ϕ**	**12**	fdej =	3,39	cm^2
negativna armatura	**2**	**ϕ**	**12**	fdej =	2,26	cm^2

strig

k = 1,6 - d	=	1,43	> 1
$\rho_l = A_{sl} / (b * d)$	=	0,0044	
$N_{sd} = 0 \Rightarrow \sigma_{cp} = 0$			
$V_{Rd} = (\tau_{Rd} * k * (1,2 + 40 * \rho_l) + 0,15 * \sigma_{cp}) * b * d$	=	34,15	kN
$V_{Rd} > V_d \Rightarrow$ potrebujemo konstr. strižno armaturo			

stremena **ϕ 8 / 20 cm**

POZ 211 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon: **L = 1,30 m**

beton C25/30	**b =**	**30**	**cm**	širina
armatura S500	**h =**	**20**	**cm**	minimalna višina

pozitivna armatura	**2**	**ϕ**	**12**
negativna armatura	**2**	**ϕ**	**12**

stremena **ϕ 8 / 20 cm**

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POZ 212 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon:	**L**	**=**	**0,90**	**m**	
beton C25/30	**b**	**=**	**20**	**cm**	širina
armatura S500	**h**	**=**	**20**	**cm**	minimalna višina
pozitivna armatura		**2**	**ϕ**	**12**	
negativna armatura		**2**	**ϕ**	**12**	
stremena		**ϕ 8 / 20 cm**			

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POZ 100 PRITLIČJE

POZ 101 KRIŽEM ARMIRANE PLOŠČE

Vpliv

stalni vpliv

lastna teža plošče	d =	22	cm	d * 25 =	5,50	kN/m^2
tlak					2,00	kN/m^2
predelne stene					1,50	kN/m^2
strop					0,25	kN/m^2
stalni vpliv skupaj				**g =**	**9,25**	**kN/m^2**
stalni vpliv od stopnic				**g_s =**	**20,00**	**kN/m**

spremenljiv vpliv:

kategorija uporabe: B	površina A > 40,0	ψ_0 =	0,70	
redukcijski faktor:		$\alpha_A = (5/7) * \psi_0 + A_0 / A$ =	0,75	
koristna obtežba		q_k =	3,00	kN/m^2
koristni vpliv skupaj		**q =** $\alpha_A * q_k$ =	**2,25**	**kN/m^2**
koristni vpliv od stopnic		**q_s =**	**6,00**	**kN/m**

Učinek vpliva izračun s programom PLATE

maksimalni moment - polje M_{sd} = 26,30 kNm

Dimenzioniranje: izvedeno s programom PLATE

lastnosti materiala:

beton C25/30

f_{ck} = 2,50 kNcm^2

geometrijski podatki:

debelina plošče:

h = 22 cm

αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2 d = h - a = 19,5 cm

τ_{Rd} = 0,030 kN/cm^2 a = 2,5 cm

armatura S500

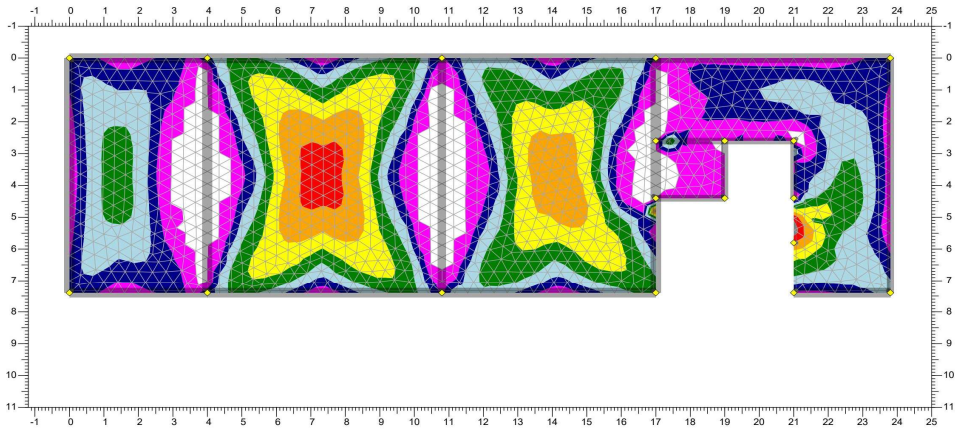
f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 =43,48 kN/cm^2

pozitivna armatura Q 636, Q 335

AMSES Plate (tm), Copyright (C) 2003-2004 Ainet projektiranje Date: 18.04.2025

File: dnevna_bol_101.plate - Plate: DNEVNA
Load case: G+Q - 1.35*G + 1.50*Q
Results: bottom A(Q)



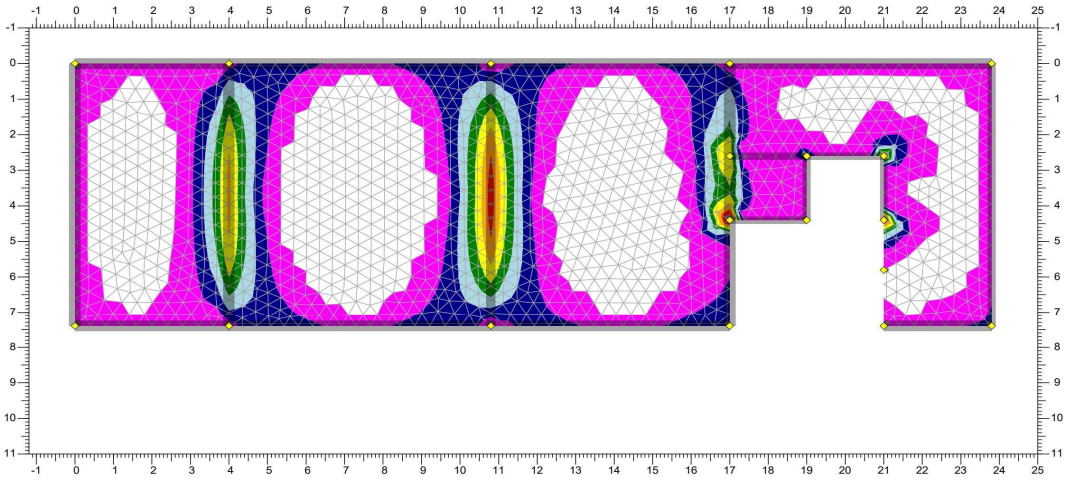


www.amses.com Unregistred copy

negativna armatura Q 636 in ojačitev ϕ 12 / 15 cm v vzdolžni smeri

AMSES Plate (tm), Copyright (C) 2003-2004 Ainet projektiranje Date: 18.04.2025

File: dnevna_bol_101.plate - Plate: DNEVNA
Load case: G+Q - 1.35*G + 1.50*Q
Results: top A(Q)





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mejno stanje uporabnosti izračun za maksimalno obremenitev v polju

RAZPOKE	$K_d = M_{sd} / \alpha f_{cd} * b * d^2 =$	0,05	$\Rightarrow$ kx =	0,145
	$x = k_x * h =$	3,19	cm	
	$z = h - a - x/3 =$	18,44	cm	
	$N_{sd} = M_{sd} / z =$	142,65	kN	
napetost jekla	$N_{sd} / A_s =$	22,43	kN/cm^2	
		224	Mpa	
načrtovana armatura:	$A_s =$	6,36	cm^2	
	premer palic:	9	mm	
	razmak palic:	10	cm	

Armatura ustreza pogoju za omejitev širine razpok iz tabel 7.2N in 7.3N iz SIST EN 1992-1-1 za razpoko širine 0,3 mm. Dovoljena širina je 0,4 mm.

POVESI

maksimalni razpon plošče:	$L_p =$	680	cm
dopusten poves:	$f_{dop} = L_p / 250 =$	2,7	cm
računski poves	$f_{max} =$	0,2	cm

dejanski poves - upoštevano lezenje

	$f_{eff} = f_{max} * (E_{cm} / E_{c,eff}) =$	0,6	cm < f_{dop}
	$E_{cm} =$	31	Gpa
	$E_{c,eff} = E_{cm} / (1 + \varphi(oo, t_0)) =$	10	Gpa
(za beton star 30 dni)	$\varphi(oo, t_{0)} =$	2	

Obvezno nadvišanje plošče za 3 cm.

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POZ 102 KRIŽEM ARMIRANE PLOŠČE

Vpliv

stalni vpliv

lastna teža plošče	d =	22	cm	d * 25 =	5,50	kN/m^2
tlak					2,00	kN/m^2
predelne stene					1,50	kN/m^2
strop					0,25	kN/m^2
stalni vpliv skupaj				**g =**	**9,25**	**kN/m^2**

spremenljiv vpliv:

kategorija uporabe: B	površina A > 40,0	ψ_0 =	0,70	
redukcijski faktor:		α_A = (5/7) *ψ_0 + A_0 / A =	0,75	
koristna obtežba		q_k =	3,00	kN/m^2
koristni vpliv skupaj		**q =** α_A * q_k =	**2,25**	**kN/m^2**

Učinek vpliva izračun s programom PLATE

maksimalni moment - polje	M_{sd} =	35,50	kNm

Dimenzioniranje: izvedeno s programom PLATE

lastnosti materiala:

beton C25/30

f_{ck} = 2,50 kNcm^2

αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2

τ_{Rd} = 0,030 kN/cm^2

armatura S500

f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 =43,48 kN/cm^2

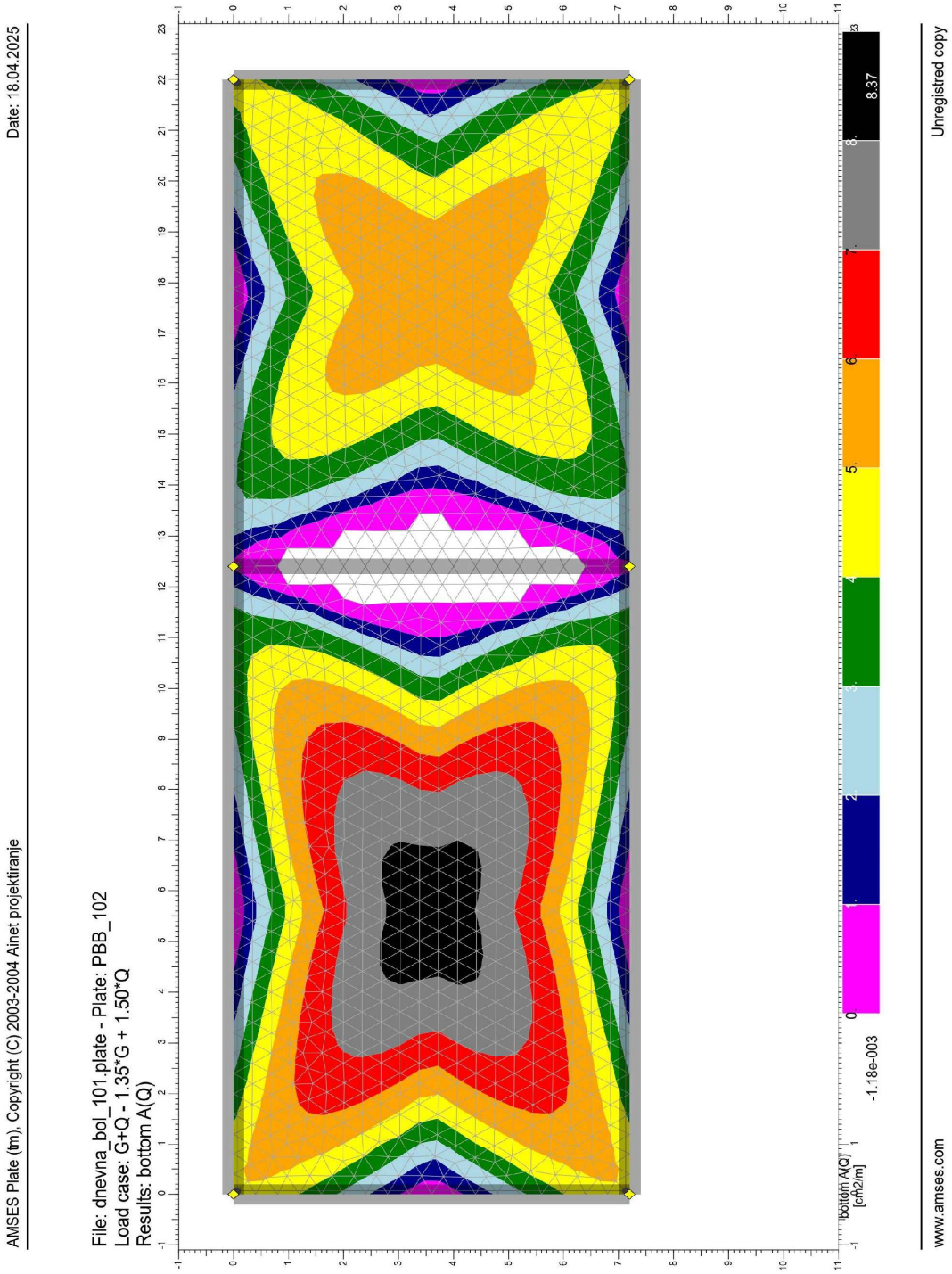
geometrijski podatki:

debelina plošče:

h =	**22**	**cm**
d = h - a =	19,5	cm
a =	2,5	cm

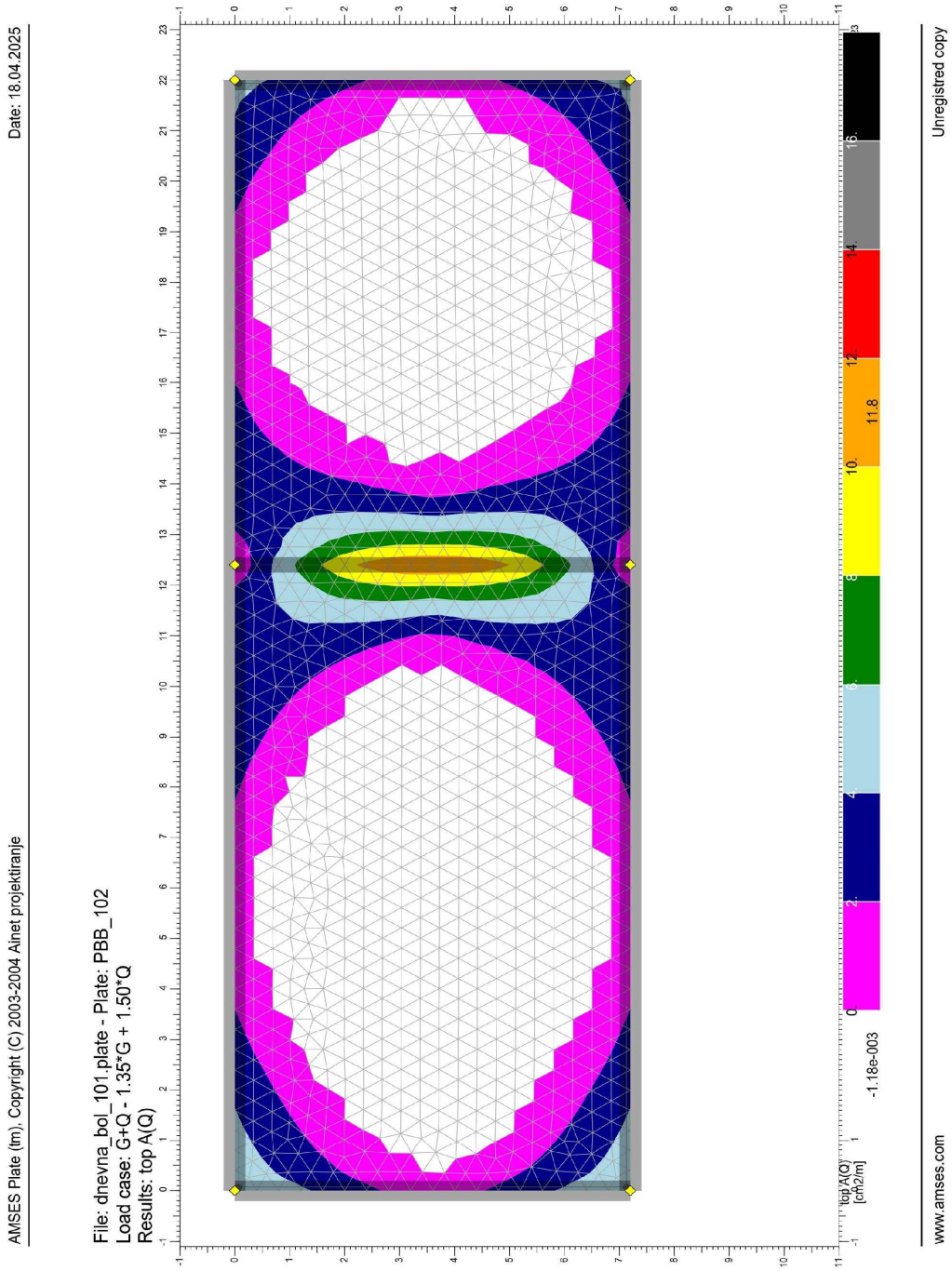
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pozitivna armatura **Q 636** **in ojačitev ϕ 12 / 15 cm v prečni smeri**



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negativna armatura Q 636 in ojačitev ϕ 12 / 15 cm v vzdolžni smeri



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mejno stanje uporabnosti izračun za maksimalno obremenitev v polju

RAZPOKE	$K_d = M_{sd} / \alpha f_{cd} * b * d^2 =$	0,07	$\Rightarrow$ kx =	0,145
	$x = k_x * h =$	3,19	cm	
	$z = h - a - x/3 =$	18,44	cm	
	$N_{sd} = M_{sd} / z =$	192,55	kN	
napetost jekla	$N_{sd} / A_s =$	13,85	kN/cm^2	
		139	Mpa	
načrtovana armatura:	$A_s =$	13,90	cm^2	
	premer palic:	9	mm	
	razmak palic:	10	cm	

Armatura ustreza pogoju za omejitev širine razpok iz tabel 7.2N in 7.3N iz SIST EN 1992-1-1 za razpoko širine 0,3 mm. Dovoljena širina je 0,4 mm.

POVESI

maksimalni razpon plošče:	$L_p =$	730	cm
dopusten poves:	$f_{dop} = L_p / 250 =$	2,9	cm
računski poves	$f_{max} =$	0,9	cm

dejanski poves - upoštevano lezenje

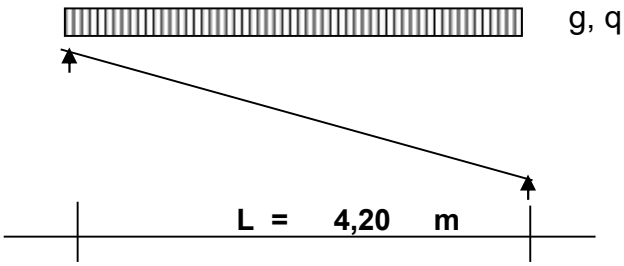
	$f_{eff} = f_{max} * (E_{cm} / E_{c,eff}) =$	2,6	cm < f_{dop}
	$E_{cm} =$	31	Gpa
	$E_{c,eff} = E_{cm} / (1 + \varphi(oo, t_0)) =$	10	Gpa
(za beton star 30 dni)	$\varphi(oo, t_{0)} =$	2	

Obvezno nadvišanje plošče za 3 cm.

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POZ 103 PROSTOLEŽEČA PLOŠČA STOPNIC

Računski model



Vpliv

lastna teža plošče	b * h * 25,00		5,00	kN/m^2
teža stopnic			2,25	kN/m^2
tlak, strop			3,00	kN/m^2
stalni vpliv skupaj		**g =**	**10,25**	**kN/m^2**
kategorija uporabe: B	površina A < 20,0	ψ_0 =	0,70	
koristna obtežba		q_k =	2,00	kN/m^2
spremenljiv vpliv skupaj		**q =** $\alpha_A * q_k$ =	**2,00**	**kN/m^2**
redukcijski faktor:		α_A = (5/7) *ψ_0 + A_0 / A =	1,00	

Učinek vpliva

maksimalni moment - polje
M_{sd} = 0,125 * (1,35 * g + 1,50 * q) * L^2 = 37,13 kNm
3713 kNcm

max prečna sila
V_d = 0,625 * (1,35 * g + 1,50 * q) * L = 44,20 kN

Dimenzioniranje:

lastnosti materiala:
beton C25/30
f_{ck} = 2,50 kNcm^2
αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2
τ_{Rd} = 0,030 kN/cm^2

geometrijski podatki:
b = 100 cm
h = 20 cm
d = 17 cm

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armatura S500

f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 =43,48 kN/cm^2

pozitivna armatura

max. moment (polje)

$K_d = M_{sd} / \alpha\, f_{cd} * b * d^2$ = 0,090 $\Rightarrow$ ks = 1,075

$A_s = k_s * M_{sd} / d * f_{yd}$ = 5,40 cm^2

vzdolžna armatura: ϕ **12** fdej = 7,54 cm^2

razmak vzdolžne armature: **e = 15 cm**

razdelilna armatura: ϕ **8**

razmak razdelilne armature: **e = 25 cm**

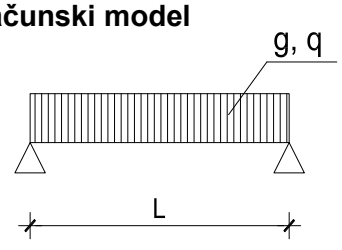
negativna armatura

vzdolžna armatura: ϕ **12** fdej = 5,65 cm^2

razmak vzdolžne armature: **e = 20 cm**

POZ 110 PROSTOLEŽEČ NOSILEC - preklada

Računski model



svetli razpon: **L_s = 2,20 m**

računski razpon: L = L_s * 1,05 = 2,31 m

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Vpliv

STALNA OBTEŽBA

lastna teža	b * h * 25,00		=	2,25	kN/m
od plošče			=	27,75	kN/m
od stene			=	9,90	kN/m
stalna skupaj		g	=	39,90	kN/m

SPREMENLJIVA OBTEŽBA

od plošče			=	6,75	kN/m
koristna skupaj		q	=	6,75	kN/m

Učinek vpliva

$M_{sd} = 0{,}125 * (1{,}35 * g + 1{,}50 * q) * L^2$ = 42,68 kNm
4268 kNcm

max prečna sila
$V_d = 0{,}500 * (1{,}35 * g + 1{,}50 * q) * L$ = 73,91 kN

Dimenzioniranje:

lastnosti materiala:

beton C25/30

$f_{ck} = 2{,}50$ kNcm^2

$\alpha f_{cd} = 0{,}85 * 2{,}50 / 1{,}50 = 1{,}42$ kN/cm^2

$\tau_{Rd} = 0{,}030$ kN/cm^2

minimalne dimenzije

b = 30 cm

h = 30 cm

d = 27 cm

armatura S500

$f_{yk} = 50{,}00$ kN/cm^2

$F_{yd} = 50{,}00 / 1{,}15 = 43{,}48$ kN/cm^2

pozitivna armatura

$K_d = M_{sd} / \alpha f_{cd} * b * d^2$ = 0,137 $\Rightarrow$ ks = 1,085

$A_s = k_s * M_{sd} / d * f_{yd}$ = 3,94 cm^2

3 ϕ 16 fdej = 6,03 cm^2

negativna armatura	**2**	**ϕ**	**12**	fdej =	2,26	cm^2

strig

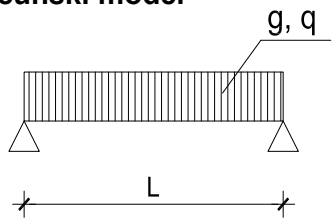
$k = 1,6 - d$	=	1,33	> 1
$\rho_l = A_{sl} / (b * d)$	=	0,0028	
$N_{sd} = 0 \Rightarrow \sigma_{cp} = 0$			
$V_{Rd} = (\tau_{Rd} * k * (1,2 + 40 * \rho_l) + 0,15 * \sigma_{cp}) * b * d$	=	48,04	kN
$V_{Rd} < V_d \Rightarrow$ potrebujemo strižno armaturo			
$V_{wd} = V_d - V_{Rd}$	=	25,87	kN
$A_{sw} / s = V_{Wd} / 0,9 * d * f_{ywd}$	=	0,0306	cm^2/cm
$A_{sw} / 2 * s =$	=	0,0153	cm^2/cm

razmak stremen s = 20 cm ⇒ Asw = 0,31 cm^2

stremena **ϕ 8 / 20 cm**

POZ 111 PROSTOLEŽEČ NOSILEC - preklada

Računski model



svetli razpon: **L_s = 3,30 m**

računski razpon: $L = L_s * 1,05$ = 3,47 m

Vpliv

STALNA OBTEŽBA

lastna teža	b * h * 25,00		=	3,75	kN/m
od plošče			=	35,15	kN/m
od stene			=	14,01	kN/m
stalna skupaj		g	=	52,91	kN/m

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SPREMENLJIVA OBTEŽBA

od stene (strehe)		=	9,40	kN/m
od plošče		=	6,75	kN/m
koristna skupaj	q	=	16,15	kN/m

Učinek vpliva

$M_{sd} = 0{,}125 * (1{,}35 * g + 1{,}50 * q) * L^2$ = 143,56 kNm
14356 kNcm

max prečna sila
$V_d = 0{,}500 * (1{,}35 * g + 1{,}50 * q) * L$ = 165,72 kN

Dimenzioniranje:

lastnosti materiala:

beton C25/30

$f_{ck} = 2{,}50$ kNcm^2

$\alpha f_{cd} = 0{,}85 * 2{,}50 / 1{,}50 = 1{,}42$ kN/cm^2

$\tau_{Rd} = 0{,}030$ kN/cm^2

minimalne dimenzije

b = 30 cm

h = 50 cm

d = 47 cm

armatura S500

$f_{yk} = 50{,}00$ kN/cm^2

$F_{yd} = 50{,}00 / 1{,}15 = 43{,}48$ kN/cm^2

pozitivna armatura

$K_d = M_{sd} / \alpha f_{cd} * b * d^2$ = 0,153 $\Rightarrow$ ks = 1,103

$A_s = k_s * M_{sd} / d * f_{yd}$ = 7,75 cm^2

	5	ϕ	**16**	fdej =	10,05	cm^2
negativna armatura	**2**	ϕ	**12**	fdej =	2,26	cm^2

strig

$k = 1{,}6 - d$	=	1,13	> 1
$\rho_l = A_{sl} / (b * d)$	=	0,0016	

$N_{sd} = 0 \Rightarrow \sigma_{cp} = 0$

$V_{Rd} = (\tau_{Rd} * k * (1,2 + 40 * \rho_l) + 0,15 * \sigma_{cp}) * b * d$ = 68,48 kN

$V_{Rd} < V_d \Rightarrow$ potrebujemo strižno armaturo

$V_{wd} = V_d - V_{Rd}$ = 97,24 kN

$A_{sw} / s = V_{Wd} / 0,9 * d * f_{ywd}$ = 0,0661 cm^2/cm

$A_{sw} / 2 * s =$ = 0,0330 cm^2/cm

razmak stremen s = 15 cm $\Rightarrow$ Asw = 0,50 cm^2

stremena **ϕ 8 / 15 cm**

POZ 112 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon: **L = 1,50 m**

beton C25/30 **b = 30 cm** širina

armatura S500 **h = 20 cm** minimalna višina

pozitivna armatura **3 ϕ 12**

negativna armatura **2 ϕ 12**

stremena **ϕ 8 / 20 cm**

POZ 113 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon: **L = 1,00 m**

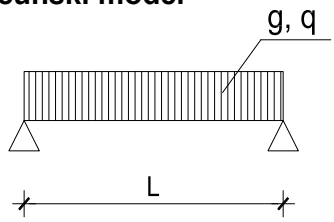
beton C25/30 **b = 30 cm** širina

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armatura S500	**h**	**=**	**20**	**cm**	minimalna višina
pozitivna armatura	**3**	**ϕ**	**12**		
negativna armatura	**2**	**ϕ**	**12**		
stremena	**ϕ 8 / 20 cm**				

POZ 114 PROSTOLEŽEČ NOSILEC - preklada

Računski model



svetli razpon: **L_s = 2,20 m**

računski razpon: $L = L_s * 1,05$ = 2,31 m

Vpliv

STALNA OBTEŽBA

lastna teža	b * h * 25,00	=	5,00	kN/m
od plošče		=	35,15	kN/m
od stene		=	14,01	kN/m
stalna skupaj	g	=	54,16	kN/m

SPREMENLJIVA OBTEŽBA

od stene (strehe)		=	9,40	kN/m
od plošče		=	6,75	kN/m
koristna skupaj	q	=	16,15	kN/m

Učinek vpliva

$M_{sd} = 0,125 * (1,35 * g + 1,50 * q) * L^2$ = 64,93 kNm

6493 kNcm

max prečna sila

$V_d = 0,500 * (1,35 * g + 1,50 * q) * L$ = 112,43 kN

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Dimenzioniranje:

lastnosti materiala:

beton C25/30

f_{ck} = 2,50 kNcm^2

αf_{cd} = 0,85 * 2,50 / 1,50 = 1,42 kN/cm^2

τ_{Rd} = 0,030 kN/cm^2

armatura S500

f_{yk} = 50,00 kN/cm^2

F_{yd} = 50,00 / 1,15 = 43,48 kN/cm^2

minimalne dimenzije

b =	**50**	**cm**
h =	**40**	**cm**
d =	37	cm

pozitivna armatura

$K_d = M_{sd} / \alpha f_{cd} * b * d^2$ = 0,067 $\Rightarrow$ ks = 1,091

$A_s = k_s * M_{sd} / d * f_{yd}$ = 4,40 cm^2

	5	ϕ	**12**	fdej =	5,65	cm^2
negativna armatura	**3**	ϕ	**12**	fdej =	3,39	cm^2

strig

k = 1,6 - d	=	1,23	> 1
$\rho_l = A_{sl} / (b * d)$	=	0,0018	
$N_{sd} = 0 \Rightarrow \sigma_{cp} = 0$			
$V_{Rd} = (\tau_{Rd} * k * (1,2 + 40 * \rho_l) + 0,15 * \sigma_{cp}) * b * d$	=	98,51	kN
$V_{Rd} < V_d \Rightarrow$ potrebujemo strižno armaturo			
$V_{wd} = V_d - V_{Rd}$	=	13,92	kN
$A_{sw} / s = V_{Wd} / 0,9 * d * f_{ywd}$	=	0,0120	cm^2/cm
$A_{sw} / 2 * s$ =	=	0,0060	cm^2/cm

razmak stremen s = 20 cm $\Rightarrow$ Asw = 0,12 cm^2

stremena **ϕ 8 / 20 cm**

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POZ 115 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon:	**L**	**=**	**1,80**	**m**	
beton C25/30	**b**	**=**	**85**	**cm**	širina
armatura S500	**h**	**=**	**20**	**cm**	minimalna višina
pozitivna armatura		**5**	**ϕ**	**12**	
negativna armatura		**4**	**ϕ**	**12**	
stremena		**ϕ 8 / 20 cm**			

POZ 116 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon:	**L**	**=**	**1,00**	**m**	
beton C25/30	**b**	**=**	**85**	**cm**	širina
armatura S500	**h**	**=**	**20**	**cm**	minimalna višina
pozitivna armatura		**5**	**ϕ**	**12**	
negativna armatura		**4**	**ϕ**	**12**	
stremena		**ϕ 8 / 20 cm**			

2083
822
0,394623

POZ 117 PROSTOLEŽEČ NOSILEC - preklada

svetli razpon:	**L**	**=**	**1,00**	**m**	
beton C25/30	**b**	**=**	**45**	**cm**	širina
armatura S500	**h**	**=**	**20**	**cm**	minimalna višina
pozitivna armatura		**3**	**ϕ**	**12**	
negativna armatura		**3**	**ϕ**	**12**	
stremena		**ϕ 8 / 20 cm**			

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VERTIKALNA KONSTRUKCIJA

ZIDANE STENE - nove

opečni zidak: skupina 2

malta: M10

debelina **d = 30 cm**

Seizmika

povratna doba: 475 let

projektni pospešek tal a_g: $a_g = 0{,}225 * g =$ 2,25 m/s^2

zemeljski pospešek tal g: $g =$ 10 m/s^2

vrsta tal: C

Horizontalne vezi v mansardi

minimalne dimenzije : **b / h = 30 / 20 cm** oziroma v širini stene

vzdolžna armatura : **+- 2 ϕ 12**

zaprta stremena: **ϕ 8 / 20 cm**

Horizontalne vezi v plošči nad pritličjem

minimalne dimenzije : **b / h = 30 / 22 cm** oziroma v širini stene

vzdolžna armatura : **+- 2 ϕ 12**

zaprta stremena: **ϕ 8 / 20 cm**

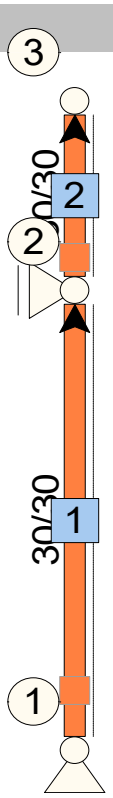
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Vertikalne vezi

Računski model

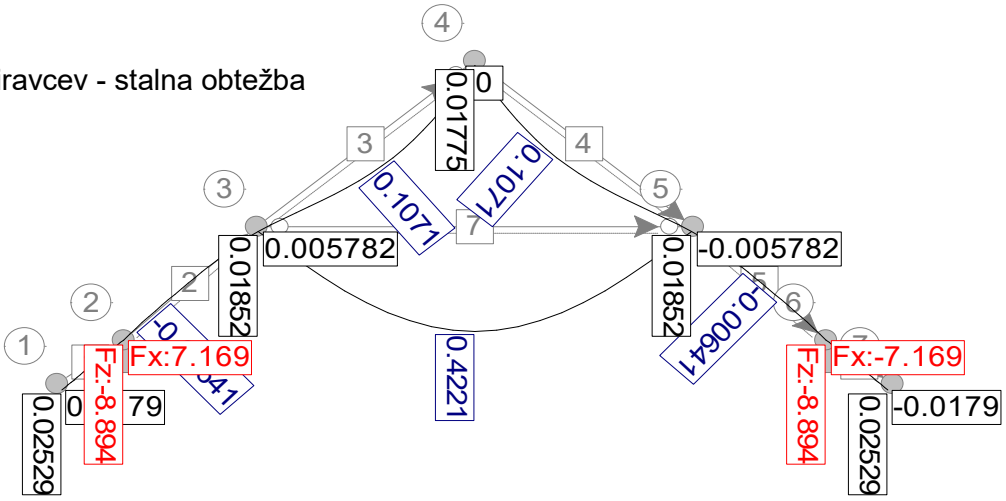
h_2 = 1,40 m

h_1 = 3,40 m



Vpliv	stalna obtežba - vertikalna	$G_v = g_v * e$ =	27,37	kN/m
	stalna obtežba - horizontalna	$G_h = g_h * e$ =	22,06	kN/m
	vplivno polje	e =	3,20	m

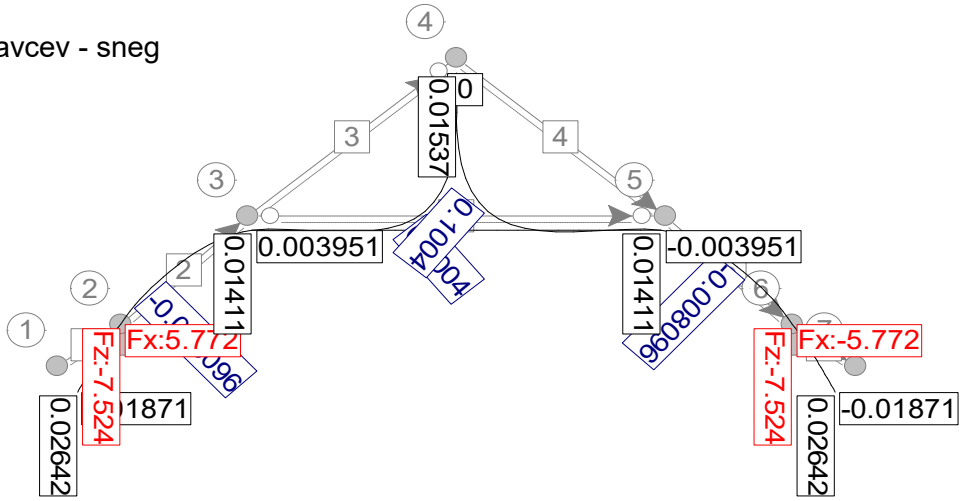
reakcije špiravcev - stalna obtežba



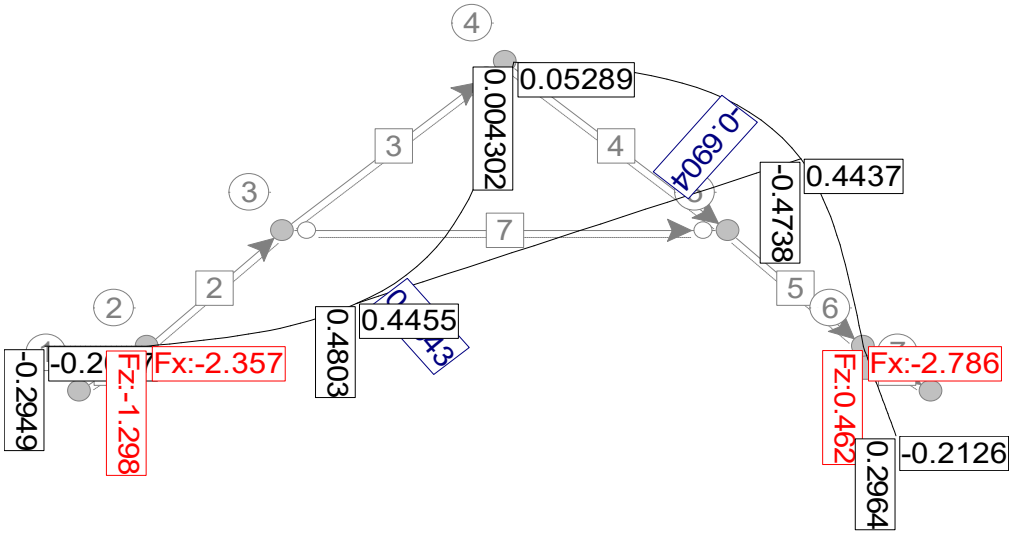
	sneg - vertikalna	$S_v = s_v * e$ =	23,14	kN/m
	sneg - vertikalna	$S_h = s_h * e$ =	17,75	kN/m
	vplivno polje	e =	3,20	m

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reakcije špiravcev - sneg



veter - vertikalna	$W_v = w_v * e =$	1,42	kN/m
veter - vertikalna	$W_h = w_h * e =$	8,57	kN/m
vplivno polje - največje	$e =$	3,20	m



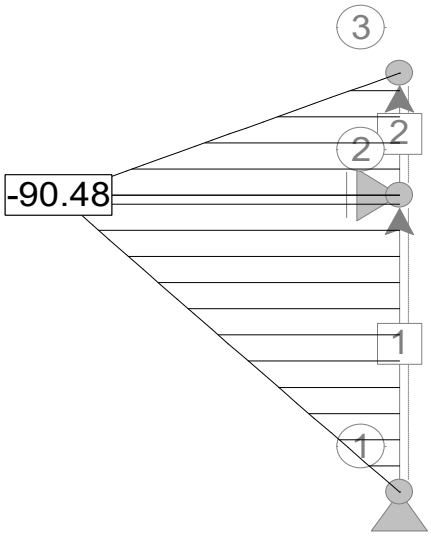
Učinek vpliva izračun s programom FRAME 2D

Nu =	-72,6	kN
Mu =	90,48	kNm

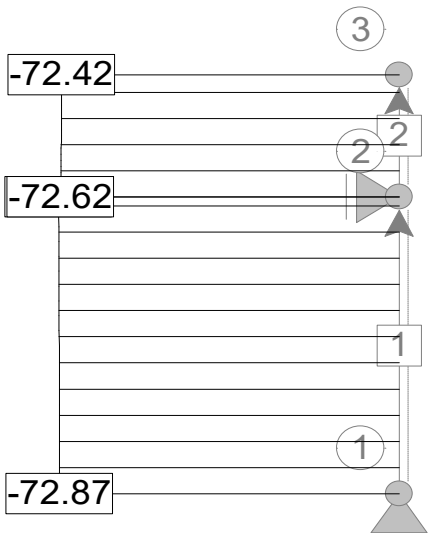
Msd = Mu - Nu * (h/2 - a) = 99,56 kNm

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projektni momenti



projektne osne sile



Dimenzioniranje **beton C25/30**
armatura S500

steber: **b = 30 cm** a = 2,5 cm

h = 30 cm d = h - a = 27,5 cm

$K_d = M_{sd} / \alpha f_{cd} * b * d^2 =$ 0,309 $\Rightarrow$ k_s = 1,145

$A_s = k_s * M_{sd} / (d * f_{yd}) + Nu / f_{yd} =$ 7,86 cm^2 / stranico

famin = 0,006 * b * h = 5,40 cm^2

vert. armatura: **4 ϕ 16**
fdej = 8,04 cm^2

stremena: **ϕ 8 / 20 cm** pritličje

ϕ 8 / 15 cm mansarda

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TEMELJENJE

Pasovni temelj pod novo steno

projektna obtežba:

od strehe - stalna	1,35	x	1,71	x	4,45		10,27 kN/m
od strehe - sprem.	1,5	x	2,12	x	4,45		14,16 kN/m
od plošče POZ 100	1,35	x	9,25	x	3,60	stalna	44,96 kN/m
od plošče POZ 100	1,5	x	2,25	x	3,60	koristna	12,15 kN/m
teža stene d =	30,00	cm, H =	4,50		(opeka)		27,03 kN/m
teža stene d =	30,00	cm, H =	0,30		(beton)		10,13 kN/m
lastna teža							11,81 kN/m
						q =	130,51 kN/m

širina: **b = 70 cm**

višina: **h = 50 cm**

faktorirana napetost : $\sigma d = q / B =$ 186,45 kN/m^2 < r_D

nosilnost: $r_D =$ 200,00 kN/m^2

Minimalna armatura::

$$A_{s,\min} = 0{,}26 \frac{f_{ctm}}{f_{yk}} b_t d = 5{,}92 \text{ cm}^2$$

$A_{s,min}$ = 0,0013 bd = 4,55 cm^2

merodajno: A_s,min = 5,92 cm^2

vzdolžna armatura +- **4 ϕ 14** fdej = 12,31 cm^2

izberem stremena **ϕ 10 / 30 cm**

priloga 1

Geometry

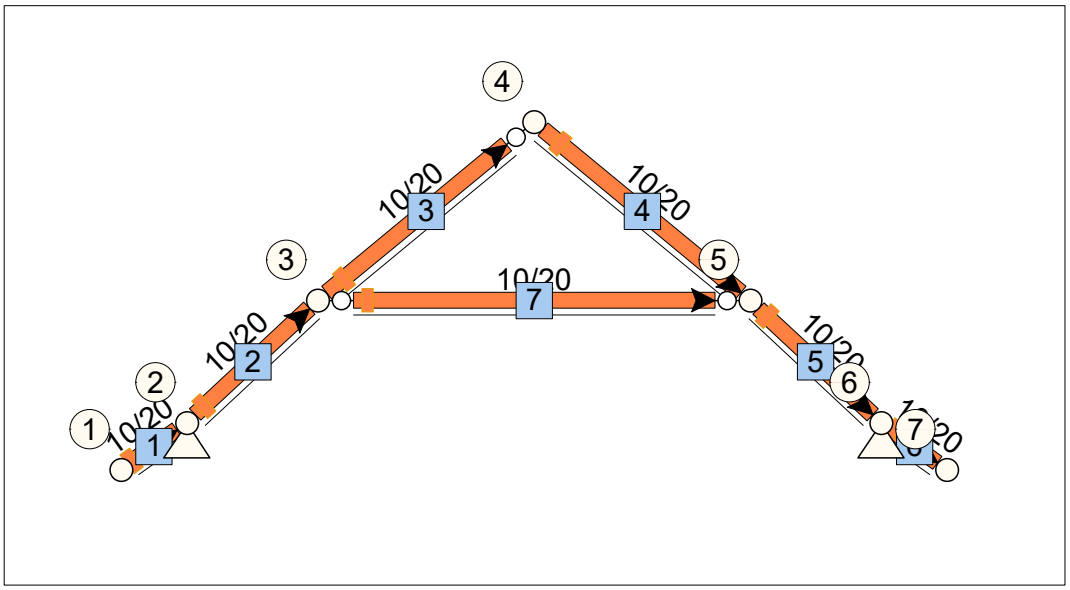


Table of Nodes

Index	X coor. [cm]	Z coor. [cm]	Supports
1:	0.00000	20.000	-
2:	70.000	-30.000	Ux Uz
3:	210.00	-160.00	-
4:	440.00	-350.00	-

Index	X coor. [cm]	Z coor. [cm]	Supports
5:	670.00	-160.00	-
6:	810.00	-30.000	Ux Uz
7:	880.00	20.000	-

Table of elements

Index	Type	Node 1	Node 2	Cross-section	Material	Length [cm]	Rel. 1	Rel. 2
1:	Non-linear	1:	2:	1: 10/20	1: C24	86.023	-	-
2:	Non-linear	2:	3:	1: 10/20	1: C24	191.05	-	-
3:	Non-linear	3:	4:	1: 10/20	1: C24	298.33	-	Ry
4:	Non-linear	4:	5:	1: 10/20	1: C24	298.33	-	-
5:	Non-linear	5:	6:	1: 10/20	1: C24	191.05	-	-
6:	Non-linear	6:	7:	1: 10/20	1: C24	86.023	-	-
7:	Non-linear	3:	5:	1: 10/20	1: C24	460.00	Ry	Ry

Table of Mateials

Index	Elastic Modulus [kN/cm2]	Coeff. of Thermal Exp. [1/d°C]
1: C24	1100.0	0.00000

Table of Cross-sections

Index	A [cm2]	Iy [cm4]	Height [cm]
1: 10/20	200.00	6666.7	20.000

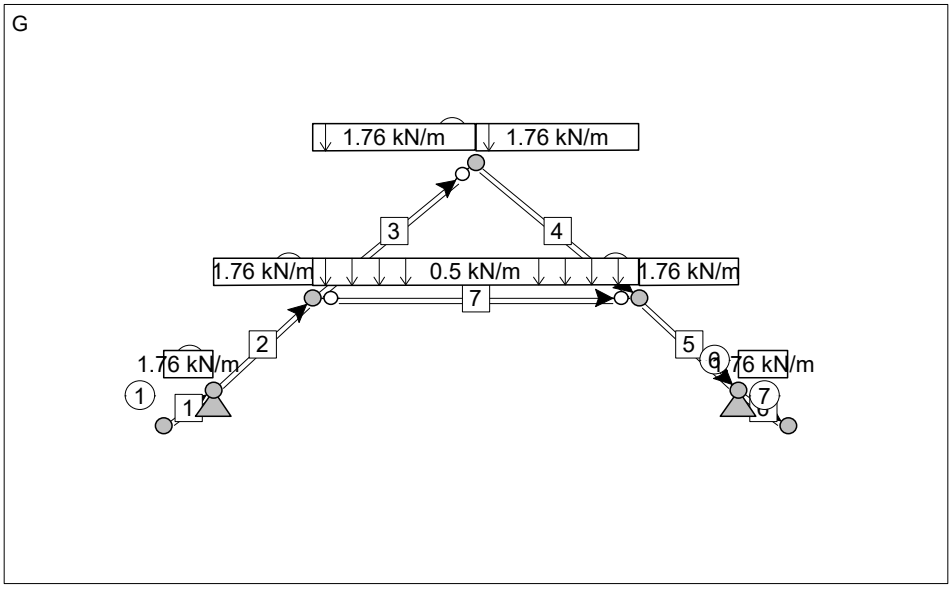
AMSES Frame2D -
Registred to: Statika Fonda

Actions & Load Cases

Load Case	Limit State	G	s	w	s1
G+Q	Ultimate	1.35	1.50	0.90	-
G+Q/ser	Service.	1.00	1.00	0.60	-
Load case: 3	Ultimate	1.35	-	0.90	1.50
Load case: 4	Ultimate	1.00	-	-	-

Load Case	Limit State	G	s	w	s1
Load case: 5	Ultimate	-	1.00	-	-
Load case: 6	Ultimate	-	-	1.00	-
Load case: 7	Ultimate	-	-	-	1.00

A1 : G

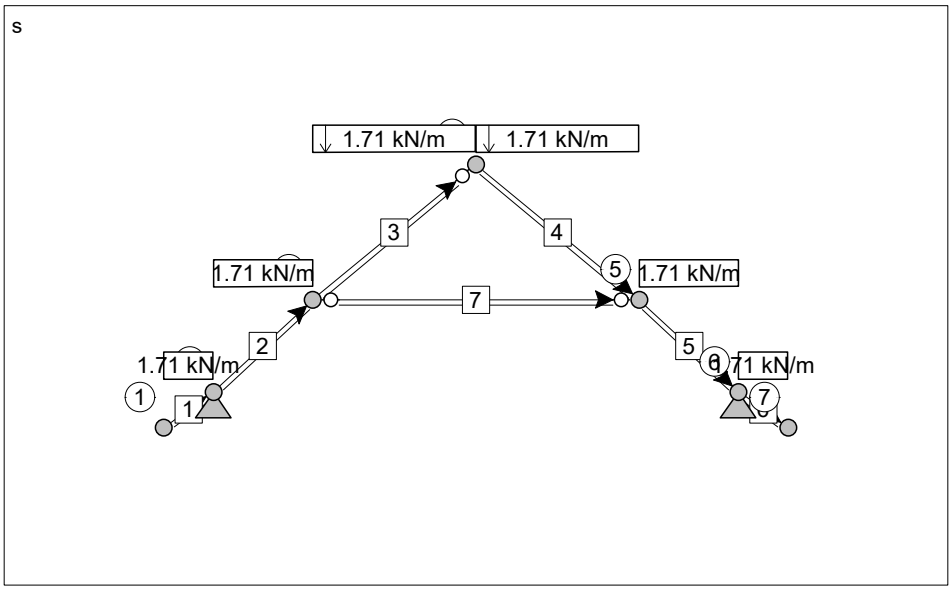


A1: G: Uniform Element Loads

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
4:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
5:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
6:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
7:	0.00000	0.00000	0.50000	0.50000	0.00000	0.00000	World	0.00	1.00	World	--

AMSES Frame2D -
Registred to: Statika Fonda

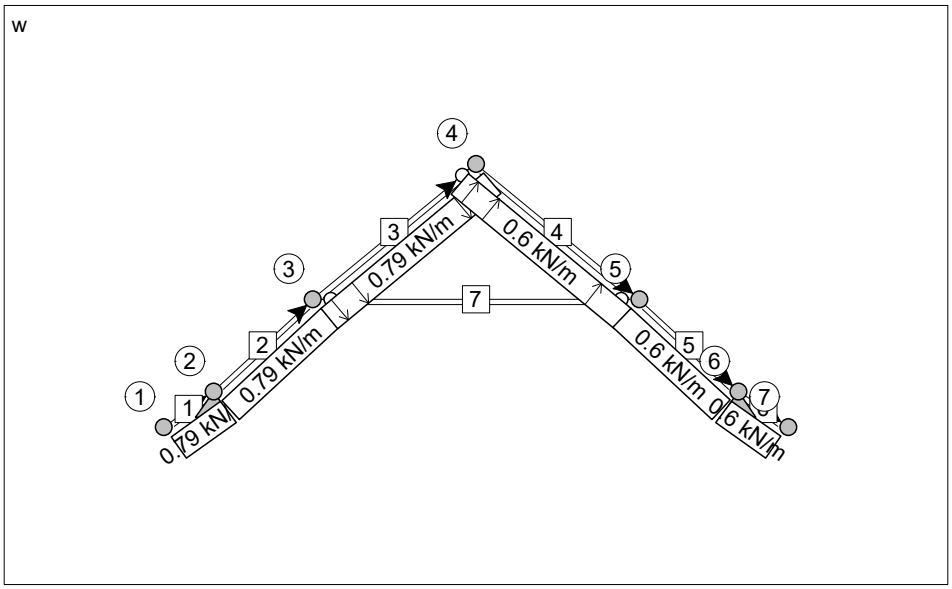
A2 : s



A2: s: Uniform Element Loads

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
4:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
5:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
6:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--

A3 : w



A3: w: Uniform Element Loads

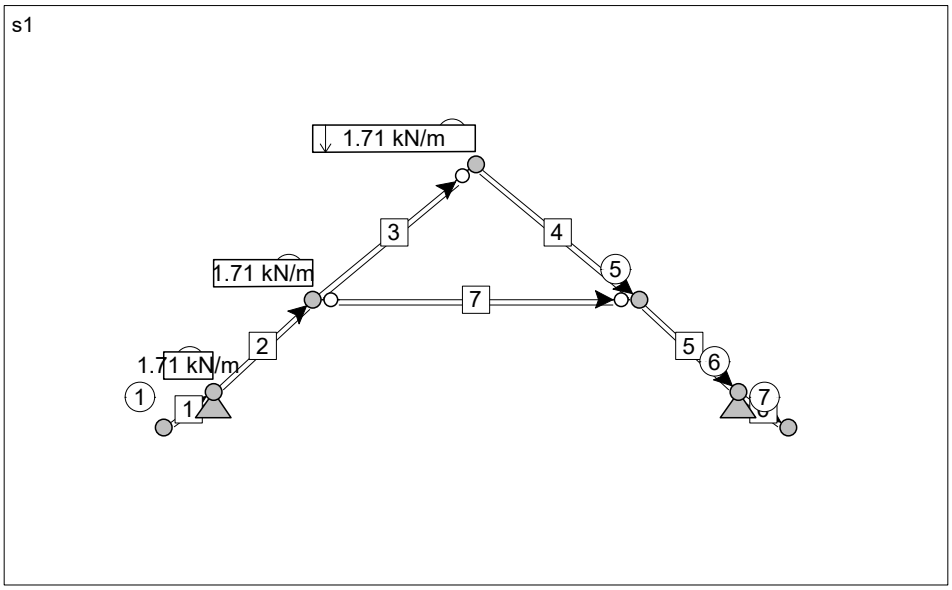
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--
2:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--

AMSES Frame2D -
Registred to: Statika Fonda

A3: w: Uniform Element Loads

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
3:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--
4:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--
5:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--
6:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--

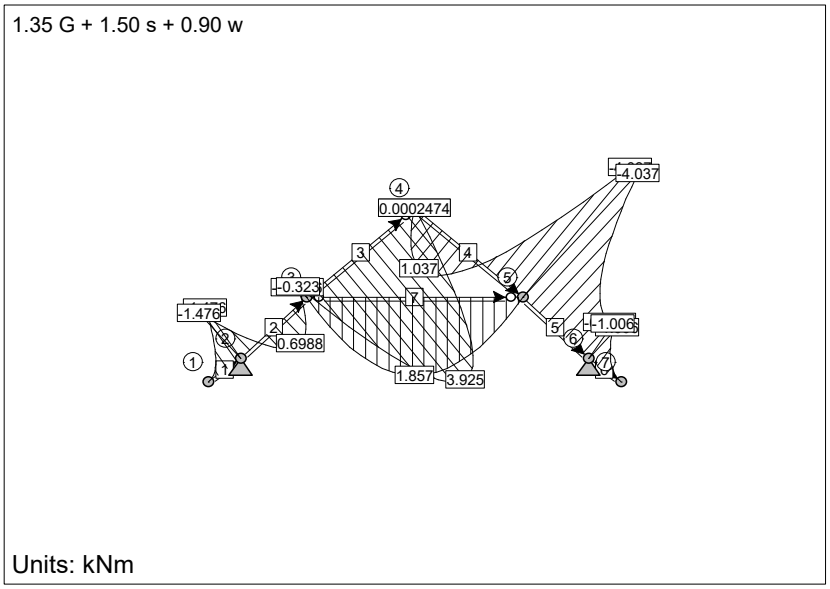
A4 : s1



A4: s1: Uniform Element Loads

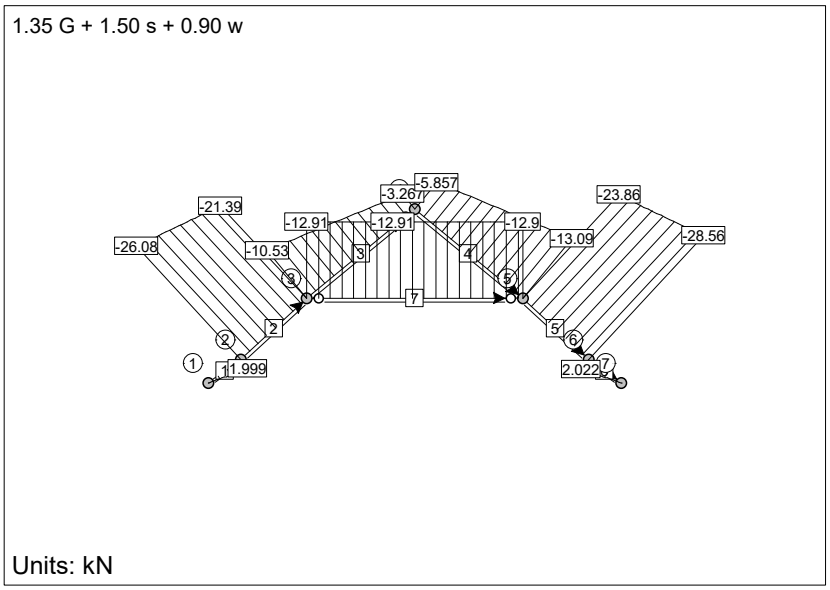
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--

LC1: G+Q: Bending Moments My

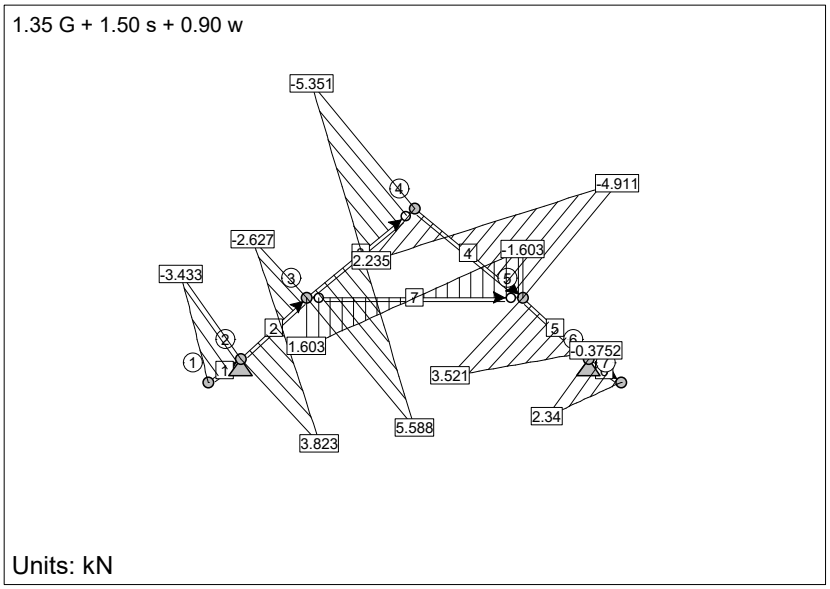


AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Axial Forces Fx



LC1: G+Q: Shear Forces Fz



LC1: G+Q: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	-0.14989	-0.20984	-0.16349
2:	-	-	-0.19656
3:	0.44064	0.50657	-0.17722
4:	0.049193	0.051540	0.061327

Node	ux [cm]	uz [cm]	fiy [deg]
5:	0.41106	-0.40768	0.0060731
6:	-	-	-0.27756
7:	-0.25757	0.35933	-0.30010

LC1: G+Q: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	16.222	-24.446	-
6:	-20.851	-22.893	-

AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.10003	-0.17159	-0.0036902
0.10	0.20005	-0.34318	-0.014760
0.15	0.30008	-0.51477	-0.033211
0.20	0.40011	-0.68636	-0.059043
0.25	0.50013	-0.85796	-0.092255
0.30	0.60015	-1.0296	-0.13285
0.35	0.70016	-1.2012	-0.18082
0.40	0.80017	-1.3728	-0.23618
0.45	0.90016	-1.5444	-0.29891
0.50	1.0001	-1.7160	-0.36903
0.55	1.1001	-1.8876	-0.44652
0.60	1.2001	-2.0592	-0.53140
0.65	1.3000	-2.2309	-0.62367
0.70	1.3999	-2.4025	-0.72331
0.75	1.4998	-2.5742	-0.83034
0.80	1.5996	-2.7459	-0.94476
0.85	1.6994	-2.9176	-1.0666
0.90	1.7992	-3.0894	-1.1957
0.95	1.8989	-3.2612	-1.3323
1.00	1.9986	-3.4330	-1.4763

LC1: G+Q: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-26.077	3.8234	-1.4763
0.05	-25.842	3.5056	-1.1263
0.10	-25.608	3.1866	-0.80664
0.15	-25.373	2.8665	-0.51756
0.20	-25.139	2.5455	-0.25909
0.25	-24.905	2.2237	-0.031333
0.30	-24.671	1.9012	0.16566
0.35	-24.437	1.5781	0.33182
0.40	-24.203	1.2546	0.46711
0.45	-23.969	0.93071	0.57148
0.50	-23.735	0.60658	0.64491
0.55	-23.500	0.28231	0.68737
0.60	-23.266	-0.042003	0.69885
0.65	-23.032	-0.36626	0.67935
0.70	-22.798	-0.69037	0.62888
0.75	-22.563	-1.0142	0.54747
0.80	-22.329	-1.3378	0.43513
0.85	-22.094	-1.6610	0.29190
0.90	-21.860	-1.9837	0.11784
0.95	-21.626	-2.3058	-0.087009
1.00	-21.391	-2.6273	-0.32257

LC1: G+Q: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-10.526	5.5883	-0.32299
0.05	-10.166	5.0429	0.47029
0.10	-9.8068	4.4960	1.1818
0.15	-9.4477	3.9479	1.8116
0.20	-9.0887	3.3990	2.3594
0.25	-8.7295	2.8495	2.8254
0.30	-8.3699	2.2996	3.2093
0.35	-8.0098	1.7496	3.5113
0.40	-7.6490	1.1997	3.7312
0.45	-7.2874	0.65009	3.8692
0.50	-6.9249	0.10089	3.9253
0.55	-6.5616	-0.44776	3.8995
0.60	-6.1975	-0.99575	3.7919
0.65	-5.8325	-1.5430	3.6026
0.70	-5.4668	-2.0894	3.3316
0.75	-5.1005	-2.6350	2.9792
0.80	-4.7338	-3.1798	2.5454
0.85	-4.3668	-3.7237	2.0305
0.90	-3.9998	-4.2668	1.4345
0.95	-3.6330	-4.8092	0.75774
1.00	-3.2667	-5.3509	0.00038388

AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-5.8575	2.2347	0.00024740
0.05	-6.2190	1.8766	0.30661
0.10	-6.5807	1.5180	0.55970
0.15	-6.9423	1.1592	0.75937
0.20	-7.3038	0.79991	0.90554
0.25	-7.6653	0.44035	0.99810
0.30	-8.0266	0.080550	1.0370
0.35	-8.3877	-0.27942	1.0222
0.40	-8.7487	-0.63948	0.95364
0.45	-9.1096	-0.99950	0.83138
0.50	-9.4704	-1.3594	0.65541

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-9.8311	-1.7189	0.42578
0.60	-10.192	-2.0780	0.14257
0.65	-10.553	-2.4365	-0.19414
0.70	-10.914	-2.7942	-0.58424
0.75	-11.275	-3.1509	-1.0276
0.80	-11.637	-3.5063	-1.5240
0.85	-12.000	-3.8603	-2.0734
0.90	-12.363	-4.2126	-2.6755
0.95	-12.727	-4.5629	-3.3300
1.00	-13.092	-4.9109	-4.0369

LC1: G+Q: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-23.856	3.5213	-4.0366
0.05	-24.090	3.3315	-3.7093
0.10	-24.324	3.1410	-3.4002
0.15	-24.558	2.9498	-3.1093
0.20	-24.793	2.7578	-2.8368
0.25	-25.027	2.5651	-2.5825
0.30	-25.262	2.3719	-2.3468
0.35	-25.498	2.1780	-2.1295
0.40	-25.733	1.9835	-1.9307
0.45	-25.968	1.7886	-1.7506
0.50	-26.204	1.5931	-1.5891

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-26.440	1.3973	-1.4463
0.60	-26.676	1.2010	-1.3222
0.65	-26.912	1.0045	-1.2169
0.70	-27.148	0.80770	-1.1303
0.75	-27.384	0.61069	-1.0626
0.80	-27.620	0.41354	-1.0136
0.85	-27.856	0.21631	-0.98356
0.90	-28.092	0.019069	-0.97231
0.95	-28.329	-0.17812	-0.97992
1.00	-28.565	-0.37519	-1.0064

LC1: G+Q: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	2.0217	2.3402	-1.0063
0.05	1.9207	2.2231	-0.90821
0.10	1.8197	2.1060	-0.81511
0.15	1.7187	1.9889	-0.72705
0.20	1.6177	1.8718	-0.64402
0.25	1.5167	1.7548	-0.56602
0.30	1.4156	1.6378	-0.49306
0.35	1.3145	1.5207	-0.42514
0.40	1.2134	1.4037	-0.36224
0.45	1.1124	1.2867	-0.30438
0.50	1.0112	1.1697	-0.25156

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.91014	1.0528	-0.20376
0.60	0.80902	0.93577	-0.16099
0.65	0.70790	0.81879	-0.12326
0.70	0.60678	0.70182	-0.090559
0.75	0.50565	0.58485	-0.062888
0.80	0.40452	0.46788	-0.040248
0.85	0.30339	0.35091	-0.022639
0.90	0.20226	0.23394	-0.010062
0.95	0.10113	0.11697	-0.0025155
1.00	0.00000	0.00000	0.00000

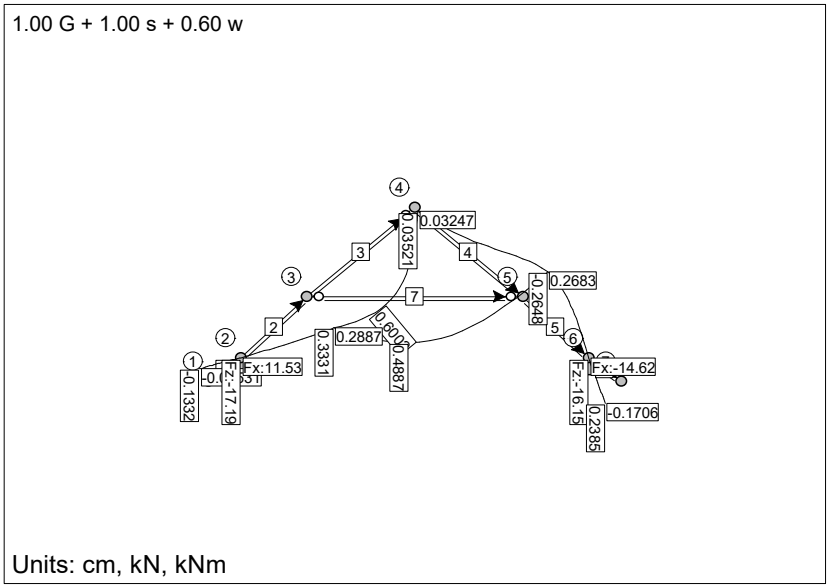
AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Element 7:

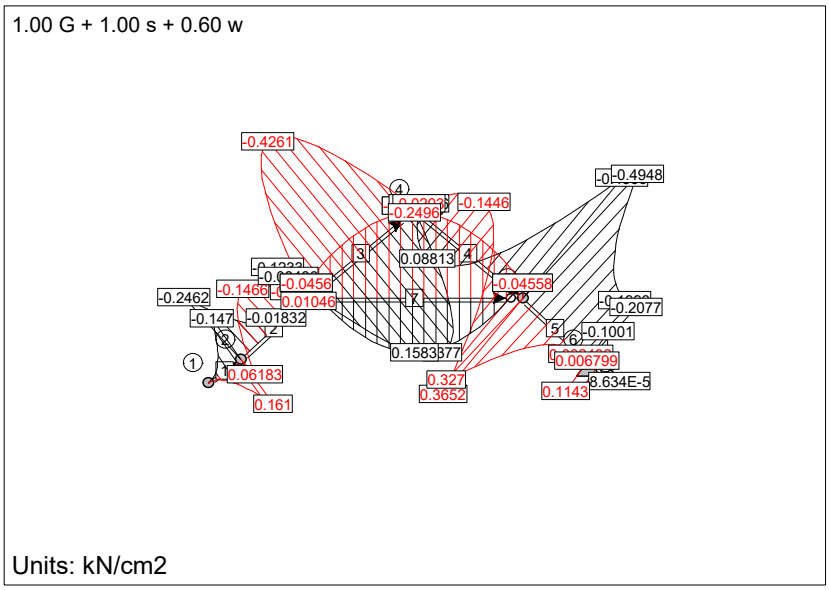
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-12.909	1.6026	0.00000
0.05	-12.909	1.4466	0.35066
0.10	-12.910	1.2893	0.66529
0.15	-12.910	1.1307	0.94359
0.20	-12.911	0.97117	1.1853
0.25	-12.911	0.81070	1.3902
0.30	-12.912	0.64946	1.5582
0.35	-12.912	0.48763	1.6889
0.40	-12.912	0.32534	1.7824
0.45	-12.912	0.16274	1.8385
0.50	-12.912	0.00000	1.8573

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-12.911	-0.16275	1.8385
0.60	-12.911	-0.32534	1.7824
0.65	-12.910	-0.48763	1.6889
0.70	-12.909	-0.64947	1.5582
0.75	-12.908	-0.81070	1.3902
0.80	-12.907	-0.97117	1.1853
0.85	-12.906	-1.1307	0.94359
0.90	-12.905	-1.2893	0.66528
0.95	-12.904	-1.4466	0.35065
1.00	-12.903	-1.6025	0.00000

LC2: G+Q/ser: Displacements and Reactions



LC2: G+Q/ser: Stress



AMSES Frame2D -
Registred to: Statika Fonda

LC2: G+Q/ser: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	-0.095314	-0.13324	-0.10336
2:	-	-	-0.12636
3:	0.28869	0.33307	-0.12021
4:	0.032470	0.035213	0.032211

Node	ux [cm]	uz [cm]	fiy [deg]
5:	0.26833	-0.26478	0.0077763
6:	-	-	-0.18327
7:	-0.17060	0.23845	-0.19928

LC2: G+Q/ser: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	11.533	-17.190	-
6:	-14.618	-16.148	-

LC2: G+Q/ser: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.070376	-0.11934	-0.0025665
0.10	0.14075	-0.23869	-0.010266
0.15	0.21113	-0.35803	-0.023099
0.20	0.28150	-0.47737	-0.041065
0.25	0.35188	-0.59672	-0.064164
0.30	0.42225	-0.71606	-0.092397
0.35	0.49262	-0.83541	-0.12576
0.40	0.56299	-0.95476	-0.16426
0.45	0.63335	-1.0741	-0.20789
0.50	0.70370	-1.1935	-0.25666

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.77405	-1.3128	-0.31056
0.60	0.84439	-1.4322	-0.36959
0.65	0.91472	-1.5516	-0.43376
0.70	0.98504	-1.6709	-0.50306
0.75	1.0553	-1.7903	-0.57750
0.80	1.1256	-1.9097	-0.65707
0.85	1.1959	-2.0291	-0.74178
0.90	1.2661	-2.1485	-0.83163
0.95	1.3364	-2.2680	-0.92661
1.00	1.4066	-2.3874	-1.0267

LC2: G+Q/ser: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-18.437	2.6058	-1.0267
0.05	-18.272	2.3843	-0.78840
0.10	-18.107	2.1623	-0.57125
0.15	-17.942	1.9397	-0.37535
0.20	-17.777	1.7167	-0.20072
0.25	-17.612	1.4933	-0.047424
0.30	-17.448	1.2695	0.084522
0.35	-17.283	1.0455	0.19509
0.40	-17.118	0.82129	0.28424
0.45	-16.953	0.59690	0.35198
0.50	-16.789	0.37240	0.39827

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-16.624	0.14784	0.42312
0.60	-16.459	-0.076737	0.42652
0.65	-16.294	-0.30128	0.40847
0.70	-16.130	-0.52574	0.36897
0.75	-15.965	-0.75008	0.30803
0.80	-15.800	-0.97425	0.22567
0.85	-15.635	-1.1982	0.12191
0.90	-15.470	-1.4220	-0.0032283
0.95	-15.305	-1.6454	-0.14972
1.00	-15.141	-1.8686	-0.31753

AMSES Frame2D -
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LC2: G+Q/ser: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-7.4409	3.9085	-0.31773
0.05	-7.1875	3.5296	0.23720
0.10	-6.9345	3.1500	0.73544
0.15	-6.6816	2.7699	1.1769
0.20	-6.4288	2.3893	1.5617
0.25	-6.1760	2.0083	1.8896
0.30	-5.9229	1.6272	2.1607
0.35	-5.6696	1.2461	2.3750
0.40	-5.4160	0.86493	2.5324
0.45	-5.1621	0.48390	2.6331
0.50	-4.9077	0.10308	2.6769

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-4.6529	-0.27749	2.6639
0.60	-4.3978	-0.65774	2.5942
0.65	-4.1422	-1.0376	2.4677
0.70	-3.8863	-1.4171	2.2846
0.75	-3.6301	-1.7963	2.0449
0.80	-3.3736	-2.1750	1.7487
0.85	-3.1171	-2.5532	1.3960
0.90	-2.8605	-2.9311	0.98698
0.95	-2.6041	-3.3087	0.52167
1.00	-2.3479	-3.6859	0.00018639

LC2: G+Q/ser: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.0558	1.6277	0.00012355
0.05	-4.3098	1.3735	0.22383
0.10	-4.5639	1.1191	0.40969
0.15	-4.8180	0.86452	0.55764
0.20	-5.0720	0.60974	0.66762
0.25	-5.3260	0.35480	0.73959
0.30	-5.5799	0.099728	0.77351
0.35	-5.8337	-0.15544	0.76937
0.40	-6.0874	-0.41066	0.72714
0.45	-6.3411	-0.66588	0.64684
0.50	-6.5947	-0.92104	0.52846

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-6.8483	-1.1761	0.37203
0.60	-7.1019	-1.4309	0.17759
0.65	-7.3556	-1.6854	-0.054836
0.70	-7.6093	-1.9396	-0.32518
0.75	-7.8632	-2.1932	-0.63339
0.80	-8.1173	-2.4463	-0.97938
0.85	-8.3717	-2.6987	-1.3631
0.90	-8.6264	-2.9502	-1.7844
0.95	-8.8816	-3.2008	-2.2432
1.00	-9.1373	-3.4503	-2.7393

LC2: G+Q/ser: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-16.781	2.4563	-2.7392
0.05	-16.946	2.3184	-2.5112
0.10	-17.110	2.1802	-2.2963
0.15	-17.275	2.0416	-2.0947
0.20	-17.440	1.9027	-1.9063
0.25	-17.605	1.7634	-1.7312
0.30	-17.770	1.6239	-1.5695
0.35	-17.935	1.4840	-1.4210
0.40	-18.101	1.3439	-1.2860
0.45	-18.266	1.2035	-1.1643
0.50	-18.431	1.0629	-1.0561

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-18.597	0.92206	-0.96129
0.60	-18.762	0.78109	-0.87995
0.65	-18.928	0.63997	-0.81208
0.70	-19.094	0.49873	-0.75770
0.75	-19.259	0.35741	-0.71680
0.80	-19.425	0.21604	-0.68941
0.85	-19.591	0.074642	-0.67553
0.90	-19.756	-0.066745	-0.67515
0.95	-19.922	-0.20809	-0.68828
1.00	-20.088	-0.34935	-0.71492

AMSES Frame2D -
Registred to: Statika Fonda

LC2: G+Q/ser: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	1.4172	1.6624	-0.71490
0.05	1.3464	1.5792	-0.64519
0.10	1.2756	1.4960	-0.57906
0.15	1.2047	1.4129	-0.51650
0.20	1.1339	1.3297	-0.45752
0.25	1.0631	1.2466	-0.40211
0.30	0.99222	1.1635	-0.35028
0.35	0.92137	1.0803	-0.30203
0.40	0.85051	0.99722	-0.25735
0.45	0.77965	0.91411	-0.21624
0.50	0.70878	0.83100	-0.17871

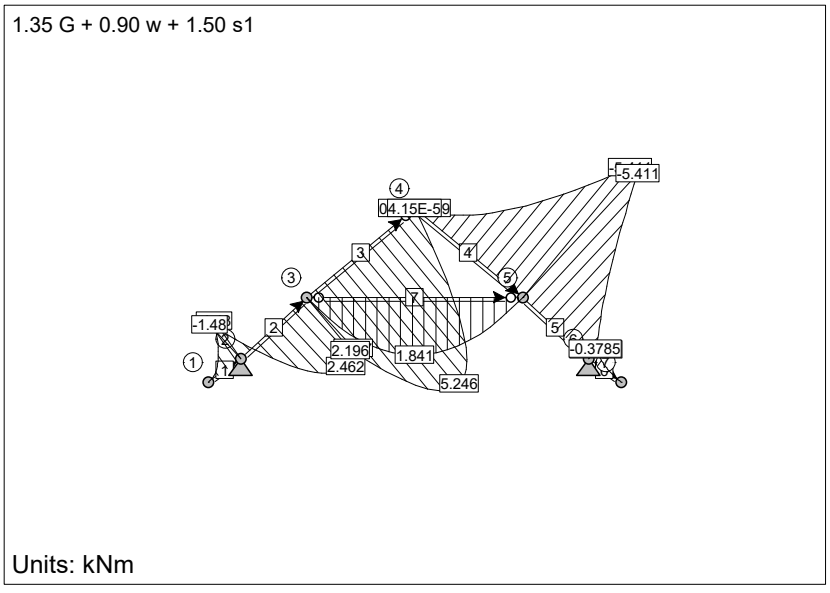
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.63791	0.74790	-0.14476
0.60	0.56703	0.66479	-0.11437
0.65	0.49616	0.58169	-0.087568
0.70	0.42528	0.49859	-0.064335
0.75	0.35440	0.41549	-0.044677
0.80	0.28352	0.33239	-0.028593
0.85	0.21264	0.24929	-0.016084
0.90	0.14176	0.16620	-0.0071482
0.95	0.070881	0.083098	-0.0017871
1.00	0.00000	0.00000	0.00000

LC2: G+Q/ser: Element 7:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-9.1198	1.1759	0.00000
0.05	-9.1200	1.0605	0.25719
0.10	-9.1203	0.94446	0.48777
0.15	-9.1206	0.82776	0.69158
0.20	-9.1209	0.71052	0.86848
0.25	-9.1212	0.59282	1.0184
0.30	-9.1214	0.47472	1.1411
0.35	-9.1216	0.35632	1.2367
0.40	-9.1217	0.23767	1.3050
0.45	-9.1217	0.11888	1.3460
0.50	-9.1216	0.00000	1.3597

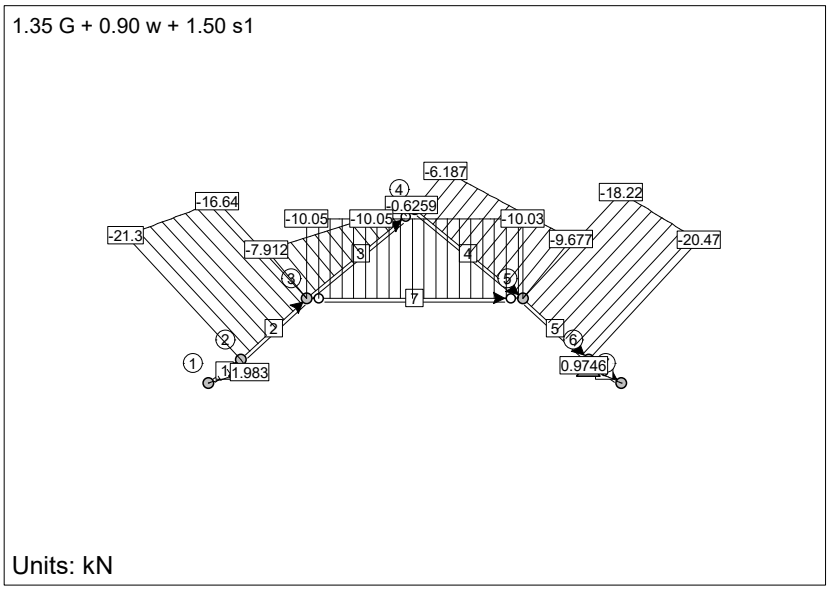
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-9.1214	-0.11888	1.3460
0.60	-9.1211	-0.23768	1.3050
0.65	-9.1207	-0.35632	1.2367
0.70	-9.1202	-0.47473	1.1411
0.75	-9.1197	-0.59282	1.0184
0.80	-9.1191	-0.71052	0.86848
0.85	-9.1185	-0.82776	0.69157
0.90	-9.1179	-0.94445	0.48777
0.95	-9.1173	-1.0605	0.25719
1.00	-9.1168	-1.1759	0.00000

LC3: Load case: 3: Bending Moments My

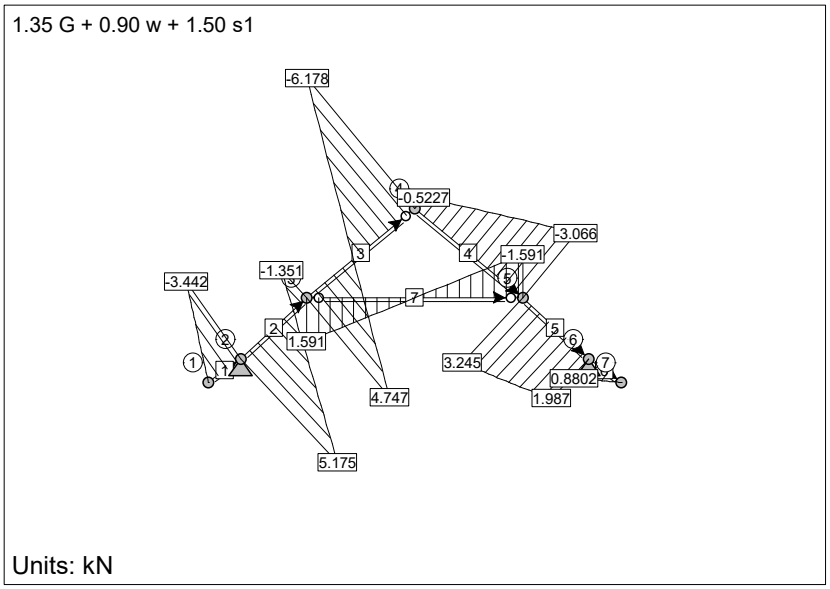


AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Axial Forces Fx



LC3: Load case: 3: Shear Forces Fz



LC3: Load case: 3: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	-0.37756	-0.53217	-0.42631
2:	-	-	-0.45947
3:	0.89378	0.99385	-0.25063
4:	0.098348	0.050311	0.37123

Node	ux [cm]	uz [cm]	fiy [deg]
5:	0.86722	-0.90263	-0.11158
6:	-	-	-0.51379
7:	-0.45664	0.63353	-0.52227

LC3: Load case: 3: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	11.881	-22.148	-
6:	-16.510	-13.904	-

AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.099239	-0.17205	-0.0037000
0.10	0.19848	-0.34409	-0.014800
0.15	0.29772	-0.51614	-0.033300
0.20	0.39695	-0.68819	-0.059200
0.25	0.49619	-0.86024	-0.092501
0.30	0.59542	-1.0323	-0.13320
0.35	0.69464	-1.2043	-0.18130
0.40	0.79386	-1.3764	-0.23680
0.45	0.89307	-1.5485	-0.29971
0.50	0.99227	-1.7205	-0.37001

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	1.0914	-1.8926	-0.44771
0.60	1.1906	-2.0647	-0.53282
0.65	1.2898	-2.2368	-0.62533
0.70	1.3889	-2.4089	-0.72524
0.75	1.4880	-2.5811	-0.83255
0.80	1.5870	-2.7532	-0.94727
0.85	1.6860	-2.9254	-1.0694
0.90	1.7850	-3.0976	-1.1989
0.95	1.8839	-3.2698	-1.3359
1.00	1.9828	-3.4421	-1.4802

LC3: Load case: 3: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-21.305	5.1750	-1.4803
0.05	-21.071	4.8552	-1.0012
0.10	-20.838	4.5340	-0.55275
0.15	-20.605	4.2116	-0.13508
0.20	-20.373	3.8881	0.25175
0.25	-20.140	3.5637	0.60763
0.30	-19.908	3.2383	0.93248
0.35	-19.675	2.9123	1.2262
0.40	-19.443	2.5856	1.4888
0.45	-19.211	2.2584	1.7201
0.50	-18.978	1.9307	1.9202

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-18.746	1.6027	2.0890
0.60	-18.513	1.2745	2.2264
0.65	-18.280	0.94605	2.3324
0.70	-18.047	0.61757	2.4071
0.75	-17.814	0.28910	2.4504
0.80	-17.581	-0.039295	2.4623
0.85	-17.347	-0.36753	2.4429
0.90	-17.113	-0.69553	2.3921
0.95	-16.879	-1.0232	2.3100
1.00	-16.645	-1.3506	2.1967

LC3: Load case: 3: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-7.9117	4.7467	2.1963
0.05	-7.5544	4.1971	2.8637
0.10	-7.1969	3.6469	3.4489
0.15	-6.8391	3.0965	3.9518
0.20	-6.4808	2.5460	4.3725
0.25	-6.1216	1.9956	4.7111
0.30	-5.7615	1.4456	4.9677
0.35	-5.4004	0.89597	5.1423
0.40	-5.0382	0.34700	5.2350
0.45	-4.6748	-0.20125	5.2459
0.50	-4.3102	-0.74870	5.1751

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-3.9445	-1.2953	5.0227
0.60	-3.5778	-1.8410	4.7889
0.65	-3.2101	-2.3858	4.4736
0.70	-2.8416	-2.9296	4.0772
0.75	-2.4725	-3.4726	3.5996
0.80	-2.1029	-4.0149	3.0411
0.85	-1.7332	-4.5564	2.4017
0.90	-1.3635	-5.0973	1.6817
0.95	-0.99430	-5.6377	0.88122
1.00	-0.62586	-6.1779	0.00038692

AMSES Frame2D -
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LC3: Load case: 3: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-6.1875	-0.52267	4.1499E-05
0.05	-6.3606	-0.65385	-0.087748
0.10	-6.5338	-0.78489	-0.19507
0.15	-6.7071	-0.91577	-0.32190
0.20	-6.8803	-1.0464	-0.46824
0.25	-7.0537	-1.1769	-0.63404
0.30	-7.2271	-1.3070	-0.81928
0.35	-7.4006	-1.4367	-1.0239
0.40	-7.5742	-1.5661	-1.2479
0.45	-7.7480	-1.6951	-1.4911
0.50	-7.9219	-1.8235	-1.7535

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-8.0960	-1.9514	-2.0351
0.60	-8.2703	-2.0786	-2.3356
0.65	-8.4449	-2.2051	-2.6551
0.70	-8.6198	-2.3309	-2.9934
0.75	-8.7950	-2.4559	-3.3504
0.80	-8.9706	-2.5800	-3.7260
0.85	-9.1465	-2.7031	-4.1200
0.90	-9.3229	-2.8252	-4.5323
0.95	-9.4998	-2.9462	-4.9628
1.00	-9.6771	-3.0659	-5.4112

LC3: Load case: 3: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-18.218	3.2448	-5.4112
0.05	-18.329	3.1873	-5.1040
0.10	-18.440	3.1292	-4.8023
0.15	-18.552	3.0705	-4.5062
0.20	-18.663	3.0113	-4.2158
0.25	-18.775	2.9514	-3.9310
0.30	-18.887	2.8910	-3.6520
0.35	-19.000	2.8300	-3.3787
0.40	-19.112	2.7685	-3.1113
0.45	-19.225	2.7064	-2.8499
0.50	-19.337	2.6437	-2.5944

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-19.450	2.5805	-2.3448
0.60	-19.563	2.5167	-2.1014
0.65	-19.676	2.4524	-1.8641
0.70	-19.789	2.3875	-1.6330
0.75	-19.903	2.3220	-1.4080
0.80	-20.016	2.2561	-1.1894
0.85	-20.130	2.1896	-0.97706
0.90	-20.243	2.1226	-0.77111
0.95	-20.357	2.0551	-0.57159
1.00	-20.470	1.9871	-0.37854

LC3: Load case: 3: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.97465	0.88017	-0.37854
0.05	0.92593	0.83614	-0.34163
0.10	0.87721	0.79212	-0.30661
0.15	0.82849	0.74810	-0.27349
0.20	0.77977	0.70408	-0.24226
0.25	0.73104	0.66007	-0.21292
0.30	0.68231	0.61605	-0.18548
0.35	0.63358	0.57204	-0.15993
0.40	0.58485	0.52804	-0.13627
0.45	0.53612	0.48403	-0.11450
0.50	0.48738	0.44002	-0.094629

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.43864	0.39602	-0.076650
0.60	0.38991	0.35201	-0.060563
0.65	0.34117	0.30801	-0.046368
0.70	0.29243	0.26401	-0.034066
0.75	0.24369	0.22001	-0.023657
0.80	0.19496	0.17601	-0.015141
0.85	0.14622	0.13200	-0.0085165
0.90	0.097478	0.088003	-0.0037851
0.95	0.048739	0.044001	-0.00094628
1.00	0.00000	0.00000	0.00000

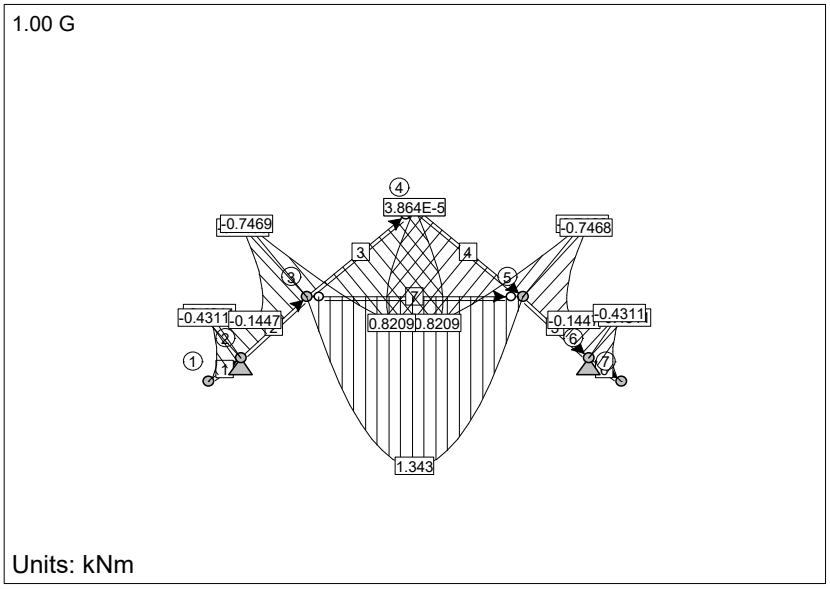
AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Element 7:

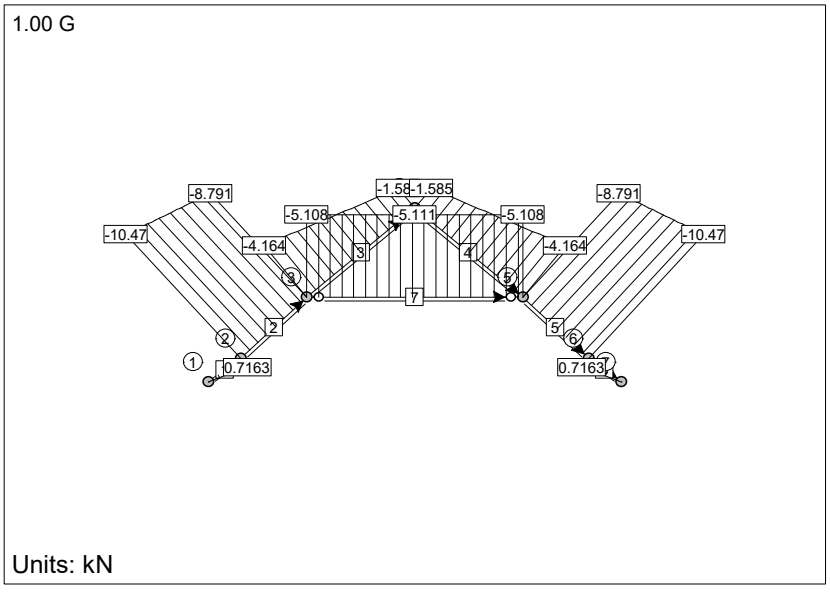
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-10.047	1.5911	0.00000
0.05	-10.047	1.4353	0.34804
0.10	-10.047	1.2785	0.66013
0.15	-10.047	1.1207	0.93604
0.20	-10.048	0.96210	1.1756
0.25	-10.048	0.80281	1.3785
0.30	-10.048	0.64295	1.5448
0.35	-10.048	0.48262	1.6742
0.40	-10.048	0.32193	1.7668
0.45	-10.047	0.16102	1.8223
0.50	-10.047	0.00000	1.8408

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-10.046	-0.16104	1.8223
0.60	-10.045	-0.32195	1.7667
0.65	-10.044	-0.48263	1.6742
0.70	-10.043	-0.64296	1.5448
0.75	-10.041	-0.80281	1.3785
0.80	-10.040	-0.96209	1.1755
0.85	-10.039	-1.1207	0.93602
0.90	-10.037	-1.2784	0.66012
0.95	-10.036	-1.4353	0.34804
1.00	-10.034	-1.5911	0.00000

LC4: Load case: 4: Bending Moments My

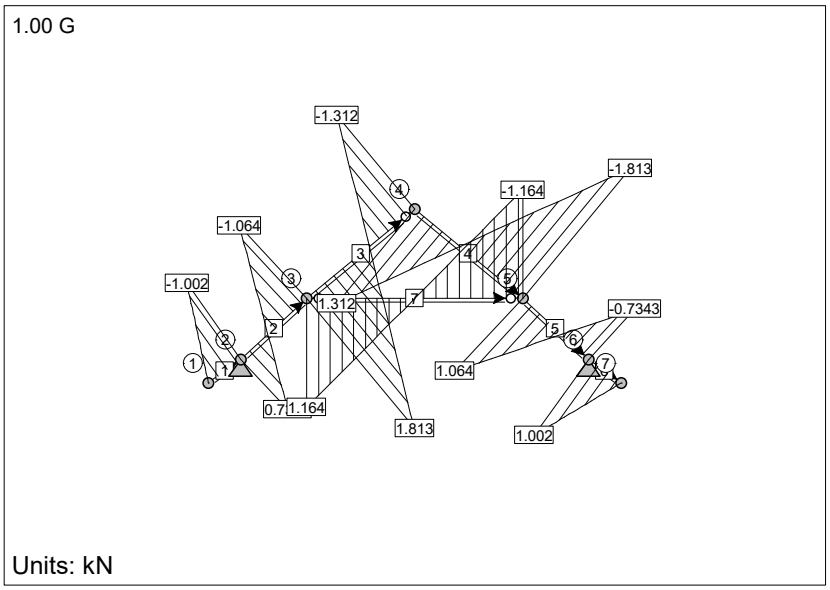


LC4: Load case: 4: Axial Forces Fx



AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Shear Forces Fz



LC4: Load case: 4: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.017900	0.025292	0.023052
2:	-	-	0.013394
3:	0.0057820	0.018521	-0.031754
4:	0.00000	0.017751	-0.062409

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-0.0057820	0.018521	0.031754
6:	-	-	-0.013394
7:	-0.017900	0.025292	-0.023052

LC4: Load case: 4: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	7.1687	-8.8940	-
6:	-7.1687	-8.8940	-

LC4: Load case: 4: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.035824	-0.050112	-0.0010777
0.10	0.071649	-0.10022	-0.0043107
0.15	0.10747	-0.15033	-0.0096992
0.20	0.14330	-0.20045	-0.017243
0.25	0.17912	-0.25056	-0.026942
0.30	0.21495	-0.30067	-0.038797
0.35	0.25077	-0.35078	-0.052807
0.40	0.28659	-0.40090	-0.068972
0.45	0.32241	-0.45101	-0.087293
0.50	0.35823	-0.50112	-0.10777

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.39405	-0.55124	-0.13040
0.60	0.42987	-0.60135	-0.15519
0.65	0.46569	-0.65147	-0.18213
0.70	0.50150	-0.70159	-0.21123
0.75	0.53731	-0.75171	-0.24249
0.80	0.57312	-0.80183	-0.27590
0.85	0.60893	-0.85196	-0.31146
0.90	0.64473	-0.90209	-0.34918
0.95	0.68053	-0.95222	-0.38906
1.00	0.71632	-1.0024	-0.43110

AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-10.467	0.73428	-0.43110
0.05	-10.383	0.64456	-0.36524
0.10	-10.299	0.55475	-0.30796
0.15	-10.215	0.46485	-0.25926
0.20	-10.131	0.37490	-0.21916
0.25	-10.048	0.28489	-0.18765
0.30	-9.9638	0.19484	-0.16474
0.35	-9.8800	0.10476	-0.15043
0.40	-9.7961	0.014671	-0.14472
0.45	-9.7123	-0.075427	-0.14762
0.50	-9.6285	-0.16552	-0.15913

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-9.5446	-0.25559	-0.17924
0.60	-9.4608	-0.34564	-0.20796
0.65	-9.3770	-0.43566	-0.24527
0.70	-9.2932	-0.52562	-0.29118
0.75	-9.2094	-0.61554	-0.34569
0.80	-9.1257	-0.70539	-0.40878
0.85	-9.0419	-0.79517	-0.48045
0.90	-8.9581	-0.88487	-0.56069
0.95	-8.8744	-0.97448	-0.64949
1.00	-8.7907	-1.0640	-0.74684

LC4: Load case: 4: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.1635	1.8126	-0.74689
0.05	-4.0345	1.6570	-0.48807
0.10	-3.9056	1.5012	-0.25251
0.15	-3.7768	1.3452	-0.040230
0.20	-3.6480	1.1890	0.14876
0.25	-3.5192	1.0327	0.31445
0.30	-3.3905	0.87626	0.45681
0.35	-3.2618	0.71979	0.57584
0.40	-3.1330	0.56328	0.67154
0.45	-3.0043	0.40675	0.74389
0.50	-2.8754	0.25023	0.79290

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-2.7466	0.093717	0.81855
0.60	-2.6176	-0.062757	0.82087
0.65	-2.4887	-0.21918	0.79984
0.70	-2.3596	-0.37555	0.75548
0.75	-2.2306	-0.53185	0.68780
0.80	-2.1015	-0.68808	0.59680
0.85	-1.9724	-0.84423	0.48251
0.90	-1.8432	-1.0003	0.34494
0.95	-1.7141	-1.1563	0.18411
1.00	-1.5850	-1.3123	3.8644E-05

LC4: Load case: 4: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.5850	1.3123	3.8644E-05
0.05	-1.7141	1.1563	0.18411
0.10	-1.8432	1.0003	0.34494
0.15	-1.9724	0.84423	0.48251
0.20	-2.1015	0.68808	0.59680
0.25	-2.2306	0.53185	0.68780
0.30	-2.3596	0.37555	0.75548
0.35	-2.4887	0.21918	0.79984
0.40	-2.6176	0.062757	0.82087
0.45	-2.7466	-0.093717	0.81855
0.50	-2.8754	-0.25023	0.79290

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-3.0043	-0.40675	0.74389
0.60	-3.1330	-0.56328	0.67154
0.65	-3.2618	-0.71979	0.57584
0.70	-3.3905	-0.87626	0.45681
0.75	-3.5192	-1.0327	0.31445
0.80	-3.6480	-1.1890	0.14876
0.85	-3.7768	-1.3452	-0.040230
0.90	-3.9056	-1.5012	-0.25251
0.95	-4.0345	-1.6570	-0.48807
1.00	-4.1635	-1.8126	-0.74689

AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-8.7907	1.0640	-0.74684
0.05	-8.8744	0.97448	-0.64949
0.10	-8.9581	0.88487	-0.56069
0.15	-9.0419	0.79517	-0.48045
0.20	-9.1257	0.70539	-0.40878
0.25	-9.2094	0.61554	-0.34569
0.30	-9.2932	0.52562	-0.29118
0.35	-9.3770	0.43566	-0.24527
0.40	-9.4608	0.34564	-0.20796
0.45	-9.5446	0.25559	-0.17924
0.50	-9.6285	0.16552	-0.15913

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-9.7123	0.075427	-0.14762
0.60	-9.7961	-0.014671	-0.14472
0.65	-9.8800	-0.10476	-0.15043
0.70	-9.9638	-0.19484	-0.16474
0.75	-10.048	-0.28489	-0.18765
0.80	-10.131	-0.37490	-0.21916
0.85	-10.215	-0.46485	-0.25926
0.90	-10.299	-0.55475	-0.30796
0.95	-10.383	-0.64456	-0.36524
1.00	-10.467	-0.73428	-0.43110

LC4: Load case: 4: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.71632	1.0024	-0.43110
0.05	0.68053	0.95222	-0.38906
0.10	0.64473	0.90209	-0.34918
0.15	0.60893	0.85196	-0.31146
0.20	0.57312	0.80183	-0.27590
0.25	0.53731	0.75171	-0.24249
0.30	0.50150	0.70159	-0.21123
0.35	0.46569	0.65147	-0.18213
0.40	0.42987	0.60135	-0.15519
0.45	0.39405	0.55124	-0.13040
0.50	0.35823	0.50112	-0.10777

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.32241	0.45101	-0.087293
0.60	0.28659	0.40090	-0.068972
0.65	0.25077	0.35078	-0.052807
0.70	0.21495	0.30067	-0.038797
0.75	0.17912	0.25056	-0.026942
0.80	0.14330	0.20045	-0.017243
0.85	0.10747	0.15033	-0.0096992
0.90	0.071649	0.10022	-0.0043107
0.95	0.035824	0.050112	-0.0010777
1.00	0.00000	0.00000	0.00000

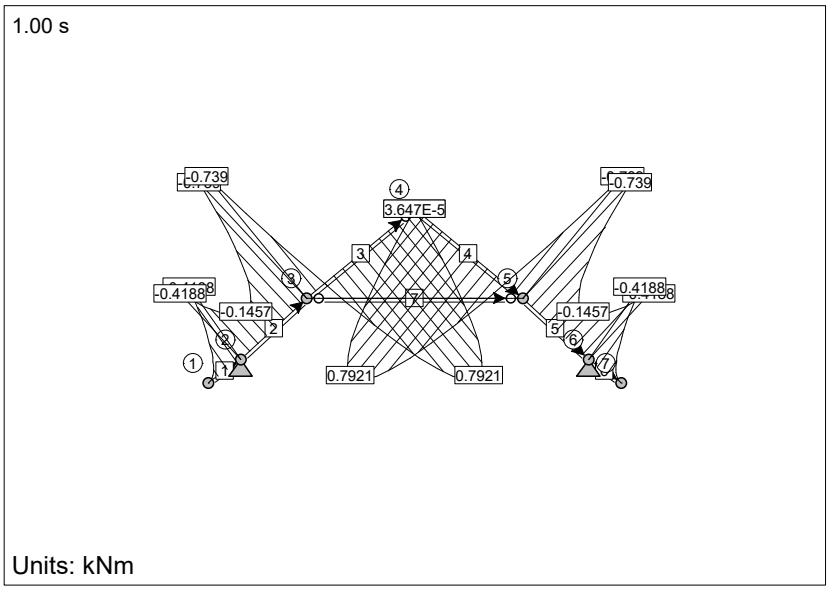
LC4: Load case: 4: Element 7:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-5.1077	1.1643	0.00000
0.05	-5.1081	1.0491	0.25455
0.10	-5.1085	0.93354	0.48256
0.15	-5.1090	0.81760	0.68394
0.20	-5.1094	0.70136	0.85862
0.25	-5.1098	0.58486	1.0065
0.30	-5.1102	0.46815	1.1276
0.35	-5.1105	0.35126	1.2219
0.40	-5.1108	0.23425	1.2892
0.45	-5.1109	0.11715	1.3296
0.50	-5.1110	0.00000	1.3431

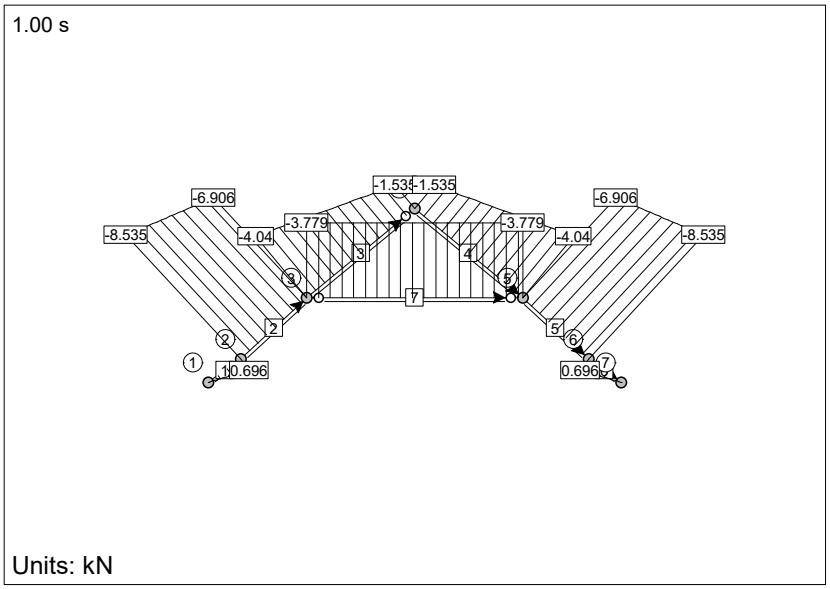
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-5.1109	-0.11715	1.3296
0.60	-5.1108	-0.23425	1.2892
0.65	-5.1105	-0.35126	1.2219
0.70	-5.1102	-0.46815	1.1276
0.75	-5.1098	-0.58486	1.0065
0.80	-5.1094	-0.70136	0.85862
0.85	-5.1090	-0.81760	0.68394
0.90	-5.1085	-0.93354	0.48256
0.95	-5.1081	-1.0491	0.25455
1.00	-5.1077	-1.1643	0.00000

AMSES Frame2D -
Registred to: Statika Fonda

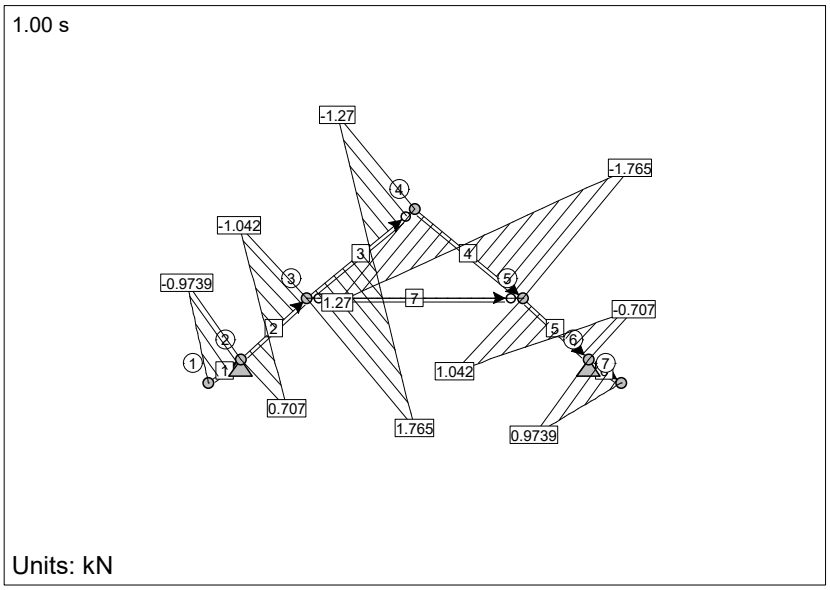
LC5: Load case: 5: Bending Moments My



LC5: Load case: 5: Axial Forces Fx



LC5: Load case: 5: Shear Forces Fz



AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.018712	0.026421	0.023910
2:	-	-	0.014526
3:	0.0039510	0.014113	-0.030309
4:	0.00000	0.015365	-0.059611
5:	-0.0039510	0.014113	0.030309
6:	-	-	-0.014526
7:	-0.018712	0.026421	-0.023910

LC5: Load case: 5: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	5.7715	-7.5240	-
6:	-5.7715	-7.5240	-

LC5: Load case: 5: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.034807	-0.048687	-0.0010471
0.10	0.069615	-0.097375	-0.0041882
0.15	0.10442	-0.14606	-0.0094235
0.20	0.13923	-0.19475	-0.016753
0.25	0.17404	-0.24344	-0.026177
0.30	0.20884	-0.29213	-0.037694
0.35	0.24365	-0.34081	-0.051306
0.40	0.27846	-0.38950	-0.067012
0.45	0.31326	-0.43819	-0.084813
0.50	0.34806	-0.48688	-0.10471
0.55	0.38287	-0.53557	-0.12670
0.60	0.41767	-0.58426	-0.15078
0.65	0.45247	-0.63296	-0.17696
0.70	0.48727	-0.68165	-0.20523
0.75	0.52206	-0.73035	-0.23559
0.80	0.55685	-0.77905	-0.26805
0.85	0.59164	-0.82775	-0.30261
0.90	0.62643	-0.87645	-0.33926
0.95	0.66121	-0.92515	-0.37801
1.00	0.69599	-0.97386	-0.41885

LC5: Load case: 5: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-8.5347	0.70699	-0.41885
0.05	-8.4532	0.61972	-0.35548
0.10	-8.3717	0.53238	-0.30045
0.15	-8.2902	0.44497	-0.25377
0.20	-8.2088	0.35752	-0.21545
0.25	-8.1273	0.27002	-0.18548
0.30	-8.0458	0.18250	-0.16386
0.35	-7.9644	0.094946	-0.15061
0.40	-7.8829	0.0073839	-0.14573
0.45	-7.8015	-0.080182	-0.14920
0.50	-7.7200	-0.16774	-0.16104
0.55	-7.6386	-0.25529	-0.18125
0.60	-7.5572	-0.34282	-0.20981
0.65	-7.4757	-0.43032	-0.24674
0.70	-7.3943	-0.51779	-0.29202
0.75	-7.3129	-0.60521	-0.34566
0.80	-7.2315	-0.69259	-0.40764
0.85	-7.1501	-0.77991	-0.47797
0.90	-7.0688	-0.86717	-0.55664
0.95	-6.9874	-0.95437	-0.64364
1.00	-6.9061	-1.0415	-0.73896

AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.0399	1.7655	-0.73900
0.05	-3.9145	1.6143	-0.48689
0.10	-3.7893	1.4629	-0.25737
0.15	-3.6641	1.3113	-0.050467
0.20	-3.5390	1.1596	0.13381
0.25	-3.4139	1.0077	0.29544
0.30	-3.2888	0.85578	0.43442
0.35	-3.1637	0.70378	0.55073
0.40	-3.0386	0.55173	0.64437
0.45	-2.9135	0.39966	0.71533
0.50	-2.7884	0.24760	0.76361

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-2.6631	0.095554	0.78921
0.60	-2.5379	-0.056460	0.79213
0.65	-2.4126	-0.20843	0.77237
0.70	-2.2872	-0.36034	0.72995
0.75	-2.1618	-0.51220	0.66487
0.80	-2.0364	-0.66398	0.57713
0.85	-1.9110	-0.81570	0.46677
0.90	-1.7855	-0.96734	0.33379
0.95	-1.6601	-1.1189	0.17820
1.00	-1.5346	-1.2704	3.6474E-05

LC5: Load case: 5: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.5346	1.2704	3.6474E-05
0.05	-1.6601	1.1189	0.17820
0.10	-1.7855	0.96734	0.33379
0.15	-1.9110	0.81570	0.46677
0.20	-2.0364	0.66398	0.57713
0.25	-2.1618	0.51220	0.66487
0.30	-2.2872	0.36034	0.72995
0.35	-2.4126	0.20843	0.77237
0.40	-2.5379	0.056460	0.79213
0.45	-2.6631	-0.095554	0.78921
0.50	-2.7884	-0.24760	0.76361

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-2.9135	-0.39966	0.71533
0.60	-3.0386	-0.55173	0.64437
0.65	-3.1637	-0.70378	0.55073
0.70	-3.2888	-0.85578	0.43442
0.75	-3.4139	-1.0077	0.29544
0.80	-3.5390	-1.1596	0.13381
0.85	-3.6641	-1.3113	-0.050467
0.90	-3.7893	-1.4629	-0.25737
0.95	-3.9145	-1.6143	-0.48689
1.00	-4.0399	-1.7655	-0.73900

LC5: Load case: 5: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-6.9061	1.0415	-0.73896
0.05	-6.9874	0.95437	-0.64364
0.10	-7.0688	0.86717	-0.55664
0.15	-7.1501	0.77991	-0.47797
0.20	-7.2315	0.69259	-0.40764
0.25	-7.3129	0.60521	-0.34566
0.30	-7.3943	0.51779	-0.29202
0.35	-7.4757	0.43032	-0.24674
0.40	-7.5572	0.34282	-0.20981
0.45	-7.6386	0.25529	-0.18125
0.50	-7.7200	0.16774	-0.16104

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-7.8015	0.080182	-0.14920
0.60	-7.8829	-0.0073839	-0.14573
0.65	-7.9644	-0.094946	-0.15061
0.70	-8.0458	-0.18250	-0.16386
0.75	-8.1273	-0.27002	-0.18548
0.80	-8.2088	-0.35752	-0.21545
0.85	-8.2902	-0.44497	-0.25377
0.90	-8.3717	-0.53238	-0.30045
0.95	-8.4532	-0.61972	-0.35548
1.00	-8.5347	-0.70699	-0.41885

AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.69599	0.97386	-0.41885
0.05	0.66121	0.92515	-0.37801
0.10	0.62643	0.87645	-0.33926
0.15	0.59164	0.82775	-0.30261
0.20	0.55685	0.77905	-0.26805
0.25	0.52206	0.73035	-0.23559
0.30	0.48727	0.68165	-0.20523
0.35	0.45247	0.63296	-0.17696
0.40	0.41767	0.58426	-0.15078
0.45	0.38287	0.53557	-0.12670
0.50	0.34806	0.48688	-0.10471

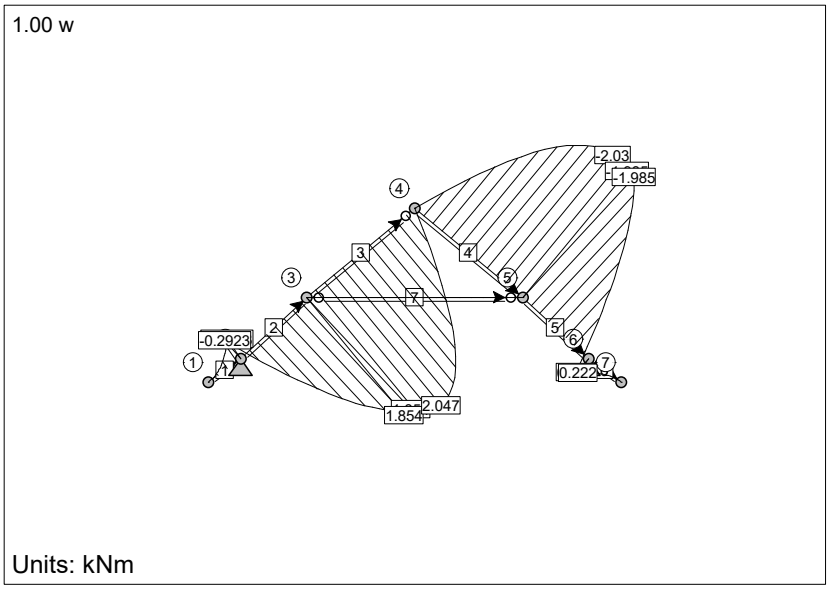
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.31326	0.43819	-0.084813
0.60	0.27846	0.38950	-0.067012
0.65	0.24365	0.34081	-0.051306
0.70	0.20884	0.29213	-0.037694
0.75	0.17404	0.24344	-0.026177
0.80	0.13923	0.19475	-0.016753
0.85	0.10442	0.14606	-0.0094235
0.90	0.069615	0.097375	-0.0041882
0.95	0.034807	0.048687	-0.0010471
1.00	0.00000	0.00000	0.00000

LC5: Load case: 5: Element 7:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-3.7793	0.00000	0.00000
0.05	-3.7793	0.00000	0.00000
0.10	-3.7793	0.00000	0.00000
0.15	-3.7793	0.00000	0.00000
0.20	-3.7793	0.00000	0.00000
0.25	-3.7793	0.00000	0.00000
0.30	-3.7793	0.00000	0.00000
0.35	-3.7793	0.00000	0.00000
0.40	-3.7793	0.00000	0.00000
0.45	-3.7793	0.00000	0.00000
0.50	-3.7793	0.00000	0.00000

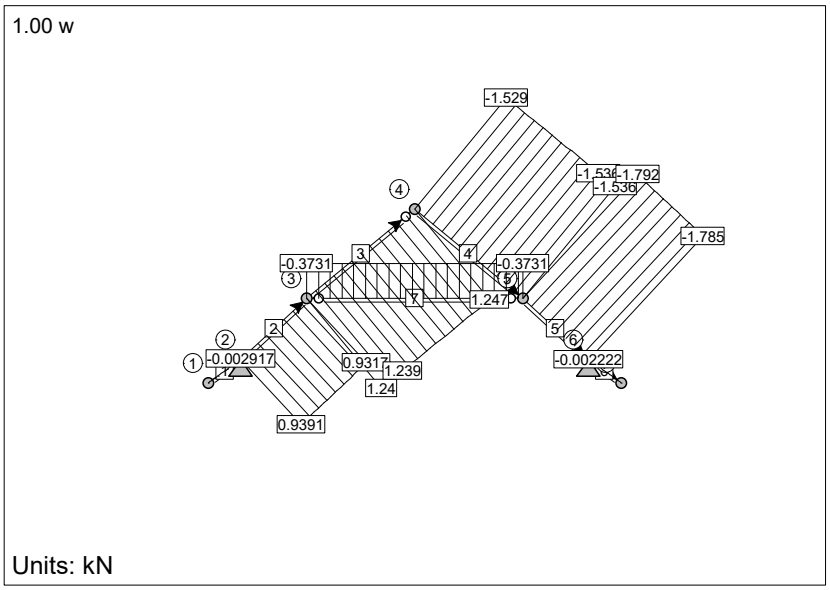
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-3.7793	0.00000	0.00000
0.60	-3.7793	0.00000	0.00000
0.65	-3.7793	0.00000	0.00000
0.70	-3.7793	0.00000	0.00000
0.75	-3.7793	0.00000	0.00000
0.80	-3.7793	0.00000	0.00000
0.85	-3.7793	0.00000	0.00000
0.90	-3.7793	0.00000	0.00000
0.95	-3.7793	0.00000	0.00000
1.00	-3.7793	0.00000	0.00000

LC6: Load case: 6: Bending Moments My

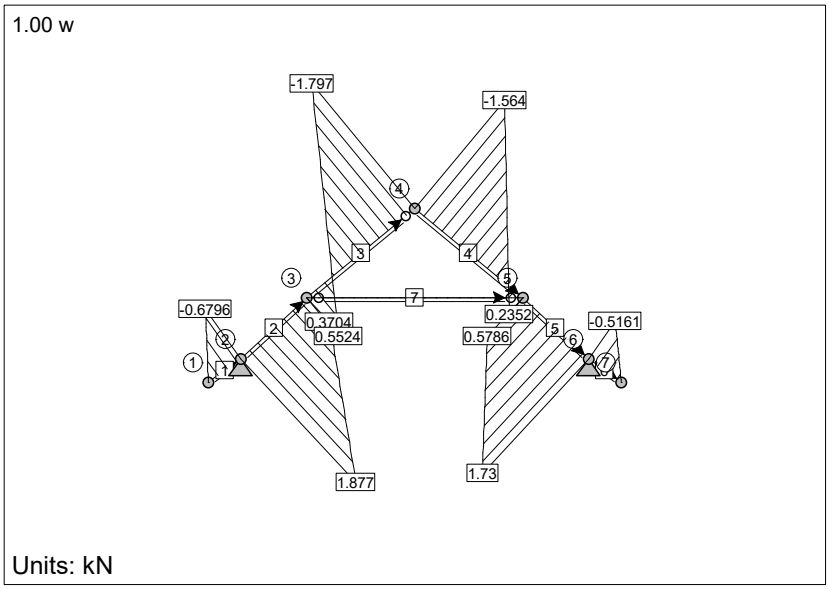


AMSES Frame2D -
Registred to: Statika Fonda

LC6: Load case: 6: Axial Forces Fx



LC6: Load case: 6: Shear Forces Fz



LC6: Load case: 6: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	-0.20971	-0.29491	-0.23939
2:	-	-	-0.24594
3:	0.44547	0.48029	-0.093582
4:	0.052895	0.0043021	0.24783

Node	ux [cm]	uz [cm]	fiy [deg]
5:	0.44370	-0.47382	-0.087759
6:	-	-	-0.24668
7:	-0.21265	0.29637	-0.24171

LC6: Load case: 6: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	-2.3573	-1.2980	-
6:	-2.7856	0.46200	-

AMSES Frame2D -
Registred to: Statika Fonda

LC6: Load case: 6: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	-0.00014197	-0.033979	-0.00073074
0.10	-0.00028395	-0.067958	-0.0029230
0.15	-0.00042595	-0.10194	-0.0065767
0.20	-0.00056800	-0.13592	-0.011692
0.25	-0.00071015	-0.16989	-0.018269
0.30	-0.00085244	-0.20387	-0.026307
0.35	-0.00099495	-0.23785	-0.035806
0.40	-0.0011377	-0.27183	-0.046768
0.45	-0.0012809	-0.30581	-0.059190
0.50	-0.0014245	-0.33979	-0.073074

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.0015688	-0.37377	-0.088420
0.60	-0.0017137	-0.40775	-0.10523
0.65	-0.0018595	-0.44173	-0.12350
0.70	-0.0020062	-0.47570	-0.14323
0.75	-0.0021541	-0.50968	-0.16442
0.80	-0.0023033	-0.54366	-0.18707
0.85	-0.0024540	-0.57764	-0.21118
0.90	-0.0026064	-0.61162	-0.23676
0.95	-0.0027607	-0.64560	-0.26380
1.00	-0.0029170	-0.67958	-0.29230

LC6: Load case: 6: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.93913	1.8772	-0.29230
0.05	0.93885	1.8017	-0.11658
0.10	0.93853	1.7262	0.051922
0.15	0.93818	1.6508	0.21322
0.20	0.93779	1.5754	0.36731
0.25	0.93738	1.5000	0.51419
0.30	0.93696	1.4246	0.65388
0.35	0.93652	1.3492	0.78636
0.40	0.93607	1.2738	0.91164
0.45	0.93563	1.1985	1.0297
0.50	0.93518	1.1232	1.1406

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.93474	1.0478	1.2443
0.60	0.93431	0.97253	1.3408
0.65	0.93390	0.89723	1.4301
0.70	0.93350	0.82195	1.5122
0.75	0.93313	0.74667	1.5871
0.80	0.93278	0.67140	1.6549
0.85	0.93245	0.59614	1.7154
0.90	0.93216	0.52089	1.7688
0.95	0.93190	0.44564	1.8149
1.00	0.93167	0.37040	1.8539

LC6: Load case: 6: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	1.2401	0.55238	1.8539
0.05	1.2397	0.43502	1.9275
0.10	1.2394	0.31767	1.9837
0.15	1.2392	0.20034	2.0223
0.20	1.2392	0.083008	2.0435
0.25	1.2392	-0.034316	2.0471
0.30	1.2393	-0.15164	2.0332
0.35	1.2395	-0.26897	2.0018
0.40	1.2398	-0.38631	1.9530
0.45	1.2402	-0.50367	1.8866
0.50	1.2406	-0.62104	1.8027

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	1.2412	-0.73844	1.7013
0.60	1.2418	-0.85586	1.5824
0.65	1.2425	-0.97332	1.4460
0.70	1.2432	-1.0908	1.2920
0.75	1.2439	-1.2083	1.1206
0.80	1.2446	-1.3259	0.93155
0.85	1.2454	-1.4436	0.72500
0.90	1.2461	-1.5612	0.50089
0.95	1.2467	-1.6790	0.25923
1.00	1.2473	-1.7968	0.00000

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LC6: Load case: 6: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.5293	-1.5637	0.00000
0.05	-1.5298	-1.4742	-0.22658
0.10	-1.5302	-1.3846	-0.43979
0.15	-1.5308	-1.2949	-0.63964
0.20	-1.5313	-1.2052	-0.82610
0.25	-1.5319	-1.1154	-0.99918
0.30	-1.5324	-1.0256	-1.1589
0.35	-1.5330	-0.93571	-1.3051
0.40	-1.5335	-0.84578	-1.4380
0.45	-1.5340	-0.75582	-1.5575
0.50	-1.5345	-0.66581	-1.6635

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-1.5349	-0.57578	-1.7561
0.60	-1.5353	-0.48572	-1.8353
0.65	-1.5356	-0.39564	-1.9010
0.70	-1.5358	-0.30554	-1.9533
0.75	-1.5360	-0.21542	-1.9921
0.80	-1.5361	-0.12530	-2.0175
0.85	-1.5361	-0.035168	-2.0295
0.90	-1.5360	0.054965	-2.0280
0.95	-1.5359	0.14510	-2.0131
1.00	-1.5357	0.23522	-1.9848

LC6: Load case: 6: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.7920	0.57859	-1.9848
0.05	-1.7918	0.63636	-1.9267
0.10	-1.7915	0.69412	-1.8632
0.15	-1.7912	0.75186	-1.7941
0.20	-1.7909	0.80958	-1.7195
0.25	-1.7905	0.86729	-1.6394
0.30	-1.7902	0.92498	-1.5538
0.35	-1.7898	0.98264	-1.4627
0.40	-1.7895	1.0403	-1.3661
0.45	-1.7891	1.0979	-1.2640
0.50	-1.7887	1.1555	-1.1564

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-1.7883	1.2131	-1.0432
0.60	-1.7879	1.2706	-0.92460
0.65	-1.7876	1.3281	-0.80048
0.70	-1.7872	1.3856	-0.67087
0.75	-1.7868	1.4431	-0.53576
0.80	-1.7865	1.5005	-0.39517
0.85	-1.7862	1.5579	-0.24910
0.90	-1.7859	1.6152	-0.097541
0.95	-1.7857	1.6726	0.059492
1.00	-1.7855	1.7298	0.22200

LC6: Load case: 6: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.0022222	-0.51613	0.22200
0.05	-0.0021050	-0.49033	0.20035
0.10	-0.0019891	-0.46452	0.17982
0.15	-0.0018742	-0.43871	0.16039
0.20	-0.0017603	-0.41291	0.14208
0.25	-0.0016472	-0.38710	0.12487
0.30	-0.0015349	-0.36129	0.10878
0.35	-0.0014233	-0.33549	0.093794
0.40	-0.0013122	-0.30968	0.079919
0.45	-0.0012017	-0.28387	0.067154
0.50	-0.0010915	-0.25807	0.055500

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.00098167	-0.23226	0.044955
0.60	-0.00087211	-0.20645	0.035520
0.65	-0.00076276	-0.18065	0.027195
0.70	-0.00065358	-0.15484	0.019980
0.75	-0.00054452	-0.12903	0.013875
0.80	-0.00043555	-0.10323	0.0088799
0.85	-0.00032663	-0.077420	0.0049950
0.90	-0.00021774	-0.051613	0.0022200
0.95	-0.00010887	-0.025807	0.00055500
1.00	0.00000	0.00000	0.00000

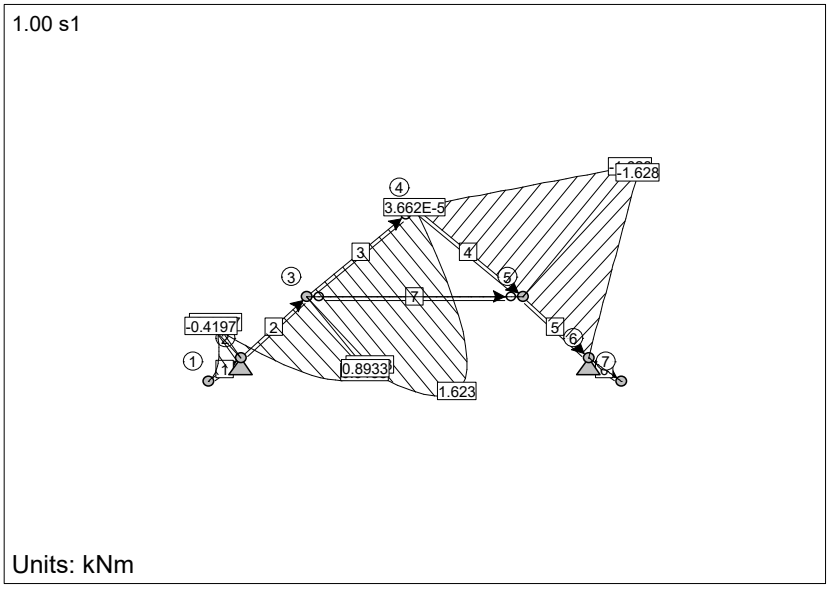
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LC6: Load case: 6: Element 7:

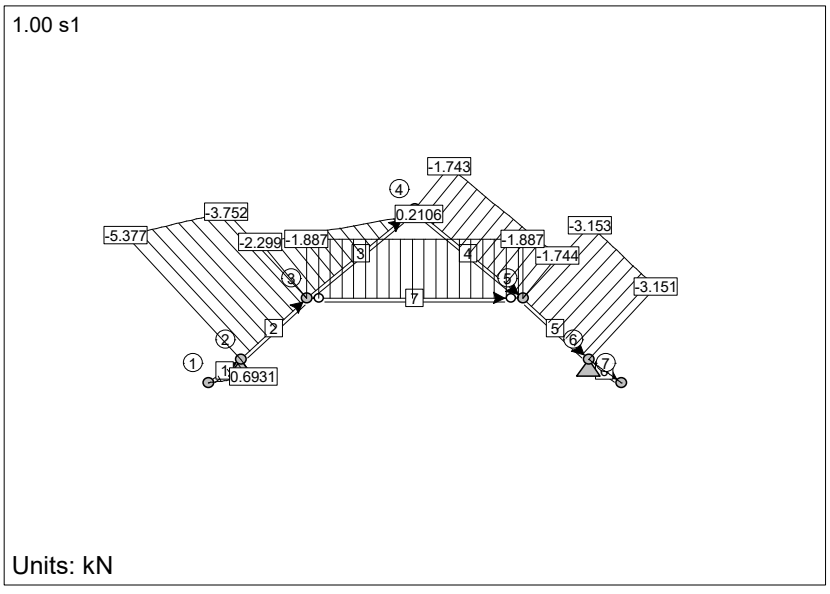
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.37308	0.00000	0.00000
0.05	-0.37308	0.00000	0.00000
0.10	-0.37308	0.00000	0.00000
0.15	-0.37308	0.00000	0.00000
0.20	-0.37308	0.00000	0.00000
0.25	-0.37308	0.00000	0.00000
0.30	-0.37308	0.00000	0.00000
0.35	-0.37308	0.00000	0.00000
0.40	-0.37308	0.00000	0.00000
0.45	-0.37308	0.00000	0.00000
0.50	-0.37308	0.00000	0.00000

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.37308	0.00000	0.00000
0.60	-0.37308	0.00000	0.00000
0.65	-0.37308	0.00000	0.00000
0.70	-0.37308	0.00000	0.00000
0.75	-0.37308	0.00000	0.00000
0.80	-0.37308	0.00000	0.00000
0.85	-0.37308	0.00000	0.00000
0.90	-0.37308	0.00000	0.00000
0.95	-0.37308	0.00000	0.00000
1.00	-0.37308	0.00000	0.00000

LC7: Load case: 7: Bending Moments My

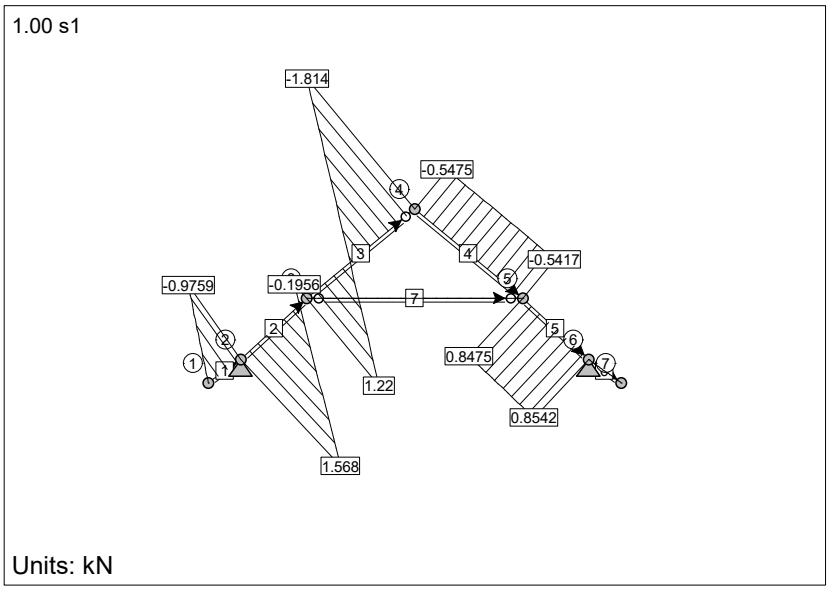


LC7: Load case: 7: Axial Forces Fx



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LC7: Load case: 7: Shear Forces Fz



LC7: Load case: 7: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	-0.12903	-0.18090	-0.14565
2:	-	-	-0.15505
3:	0.29789	0.32741	-0.077768
4:	0.032341	0.0092364	0.14279

Node	ux [cm]	uz [cm]	fiy [deg]
5:	0.29350	-0.31129	-0.047236
6:	-	-	-0.16887
7:	-0.14767	0.20610	-0.16887

LC7: Load case: 7: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	2.8861	-5.9971	-
6:	-2.8861	-1.5269	-

LC7: Load case: 7: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.034663	-0.048790	-0.0010493
0.10	0.069326	-0.097580	-0.0041971
0.15	0.10399	-0.14637	-0.0094434
0.20	0.13865	-0.19516	-0.016788
0.25	0.17332	-0.24395	-0.026232
0.30	0.20798	-0.29274	-0.037774
0.35	0.24264	-0.34153	-0.051415
0.40	0.27730	-0.39032	-0.067154
0.45	0.31196	-0.43912	-0.084992
0.50	0.34662	-0.48791	-0.10493

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.38128	-0.53670	-0.12696
0.60	0.41594	-0.58550	-0.15110
0.65	0.45059	-0.63429	-0.17733
0.70	0.48525	-0.68309	-0.20566
0.75	0.51990	-0.73189	-0.23609
0.80	0.55455	-0.78069	-0.26862
0.85	0.58919	-0.82949	-0.30325
0.90	0.62383	-0.87830	-0.33998
0.95	0.65847	-0.92711	-0.37880
1.00	0.69310	-0.97592	-0.41973

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LC7: Load case: 7: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-5.3769	1.5682	-0.41973
0.05	-5.2956	1.4805	-0.27412
0.10	-5.2144	1.3927	-0.13689
0.15	-5.1331	1.3048	-0.0080464
0.20	-5.0519	1.2169	0.11239
0.25	-4.9708	1.1288	0.22442
0.30	-4.8896	1.0407	0.32804
0.35	-4.8084	0.95251	0.42324
0.40	-4.7273	0.86429	0.51001
0.45	-4.6461	0.77603	0.58836
0.50	-4.5649	0.68774	0.65827

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-4.4838	0.59942	0.71975
0.60	-4.4026	0.51108	0.77279
0.65	-4.3214	0.42273	0.81739
0.70	-4.2401	0.33438	0.85355
0.75	-4.1589	0.24603	0.88127
0.80	-4.0776	0.15768	0.90055
0.85	-3.9963	0.069343	0.91139
0.90	-3.9150	-0.018978	0.91380
0.95	-3.8337	-0.10728	0.90777
1.00	-3.7524	-0.19555	0.89331

LC7: Load case: 7: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-2.2990	1.2201	0.89327
0.05	-2.1742	1.0679	1.0639
0.10	-2.0493	0.91565	1.2119
0.15	-1.9244	0.76342	1.3371
0.20	-1.7994	0.61122	1.4396
0.25	-1.6744	0.45906	1.5194
0.30	-1.5493	0.30695	1.5766
0.35	-1.4240	0.15491	1.6110
0.40	-1.2987	0.0029389	1.6228
0.45	-1.1733	-0.14895	1.6119
0.50	-1.0478	-0.30074	1.5784

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.92214	-0.45245	1.5222
0.60	-0.79643	-0.60407	1.4434
0.65	-0.67065	-0.75560	1.3420
0.70	-0.54480	-0.90704	1.2180
0.75	-0.41890	-1.0584	1.0714
0.80	-0.29297	-1.2097	0.90222
0.85	-0.16703	-1.3610	0.71049
0.90	-0.041100	-1.5122	0.49620
0.95	0.084789	-1.6633	0.25938
1.00	0.21061	-1.8145	3.6620E-05

LC7: Load case: 7: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.7427	-0.54753	0.00000
0.05	-1.7427	-0.54751	-0.081670
0.10	-1.7427	-0.54747	-0.16334
0.15	-1.7427	-0.54740	-0.24499
0.20	-1.7428	-0.54730	-0.32664
0.25	-1.7428	-0.54717	-0.40827
0.30	-1.7428	-0.54701	-0.48987
0.35	-1.7429	-0.54682	-0.57145
0.40	-1.7430	-0.54660	-0.65300
0.45	-1.7431	-0.54636	-0.73452
0.50	-1.7431	-0.54608	-0.81599

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-1.7432	-0.54578	-0.89743
0.60	-1.7433	-0.54545	-0.97881
0.65	-1.7435	-0.54508	-1.0601
0.70	-1.7436	-0.54469	-1.1414
0.75	-1.7437	-0.54427	-1.2226
0.80	-1.7438	-0.54383	-1.3038
0.85	-1.7440	-0.54335	-1.3849
0.90	-1.7442	-0.54284	-1.4659
0.95	-1.7443	-0.54231	-1.5468
1.00	-1.7445	-0.54175	-1.6277

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LC7: Load case: 7: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-3.1532	0.84751	-1.6277
0.05	-3.1530	0.84817	-1.5467
0.10	-3.1528	0.84878	-1.4656
0.15	-3.1527	0.84937	-1.3845
0.20	-3.1525	0.84992	-1.3034
0.25	-3.1524	0.85044	-1.2222
0.30	-3.1522	0.85092	-1.1409
0.35	-3.1521	0.85138	-1.0596
0.40	-3.1520	0.85179	-0.97824
0.45	-3.1519	0.85218	-0.89685
0.50	-3.1518	0.85253	-0.81543

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-3.1517	0.85285	-0.73398
0.60	-3.1517	0.85313	-0.65250
0.65	-3.1516	0.85339	-0.57099
0.70	-3.1515	0.85360	-0.48947
0.75	-3.1515	0.85379	-0.40792
0.80	-3.1514	0.85394	-0.32635
0.85	-3.1514	0.85405	-0.24478
0.90	-3.1514	0.85414	-0.16319
0.95	-3.1514	0.85419	-0.081596
1.00	-3.1514	0.85421	0.00000

LC7: Load case: 7: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.00000	0.00000	0.00000
0.10	0.00000	0.00000	0.00000
0.15	0.00000	0.00000	0.00000
0.20	0.00000	0.00000	0.00000
0.25	0.00000	0.00000	0.00000
0.30	0.00000	0.00000	0.00000
0.35	0.00000	0.00000	0.00000
0.40	0.00000	0.00000	0.00000
0.45	0.00000	0.00000	0.00000
0.50	0.00000	0.00000	0.00000

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.00000	0.00000	0.00000
0.60	0.00000	0.00000	0.00000
0.65	0.00000	0.00000	0.00000
0.70	0.00000	0.00000	0.00000
0.75	0.00000	0.00000	0.00000
0.80	0.00000	0.00000	0.00000
0.85	0.00000	0.00000	0.00000
0.90	0.00000	0.00000	0.00000
0.95	0.00000	0.00000	0.00000
1.00	0.00000	0.00000	0.00000

LC7: Load case: 7: Element 7:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.8874	0.00000	0.00000
0.05	-1.8874	0.00000	0.00000
0.10	-1.8874	0.00000	0.00000
0.15	-1.8874	0.00000	0.00000
0.20	-1.8874	0.00000	0.00000
0.25	-1.8874	0.00000	0.00000
0.30	-1.8874	0.00000	0.00000
0.35	-1.8874	0.00000	0.00000
0.40	-1.8874	0.00000	0.00000
0.45	-1.8874	0.00000	0.00000
0.50	-1.8874	0.00000	0.00000

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-1.8874	0.00000	0.00000
0.60	-1.8874	0.00000	0.00000
0.65	-1.8874	0.00000	0.00000
0.70	-1.8874	0.00000	0.00000
0.75	-1.8874	0.00000	0.00000
0.80	-1.8874	0.00000	0.00000
0.85	-1.8874	0.00000	0.00000
0.90	-1.8874	0.00000	0.00000
0.95	-1.8874	0.00000	0.00000
1.00	-1.8874	0.00000	0.00000

Element 5 design results (Building code - EC5):

Cross-section [cm]
[10/20], h = 20.00, b = 10.00

Element supports width left = 0.00 cm, right = 0.00 cm

Timber: C24 (solid)

Supports and buckling fields around y axis

Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

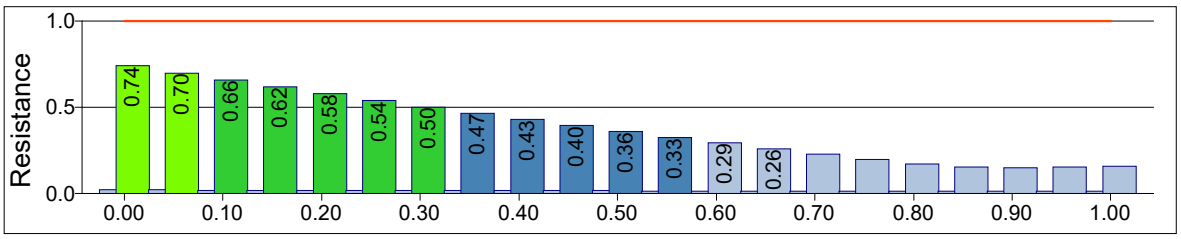
Supports and buckling fields around z axis

Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

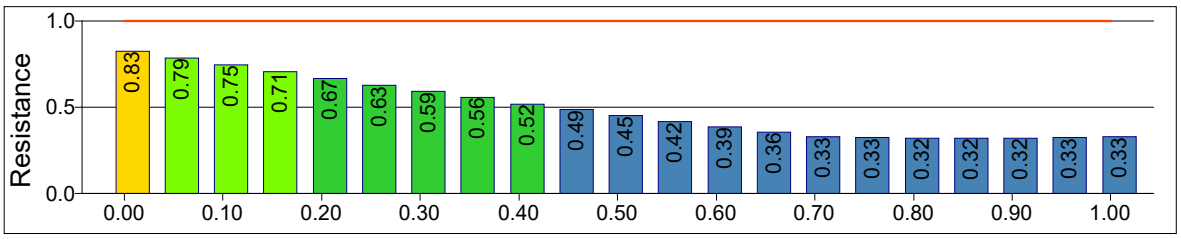
Element service class 2

Ultimate Limit States

Shear & CS Resistance

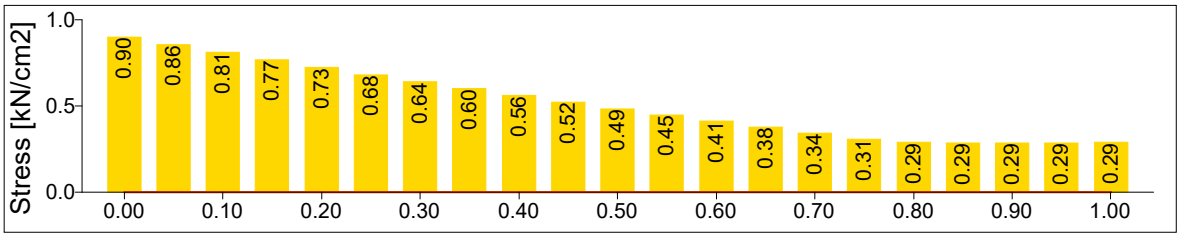


Stability Resistance

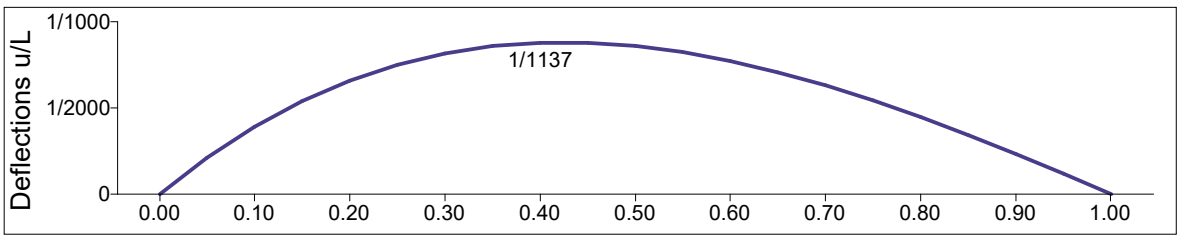


Serviceability Limit States

Serviceability stresses



Element deflections (local)



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Geometry

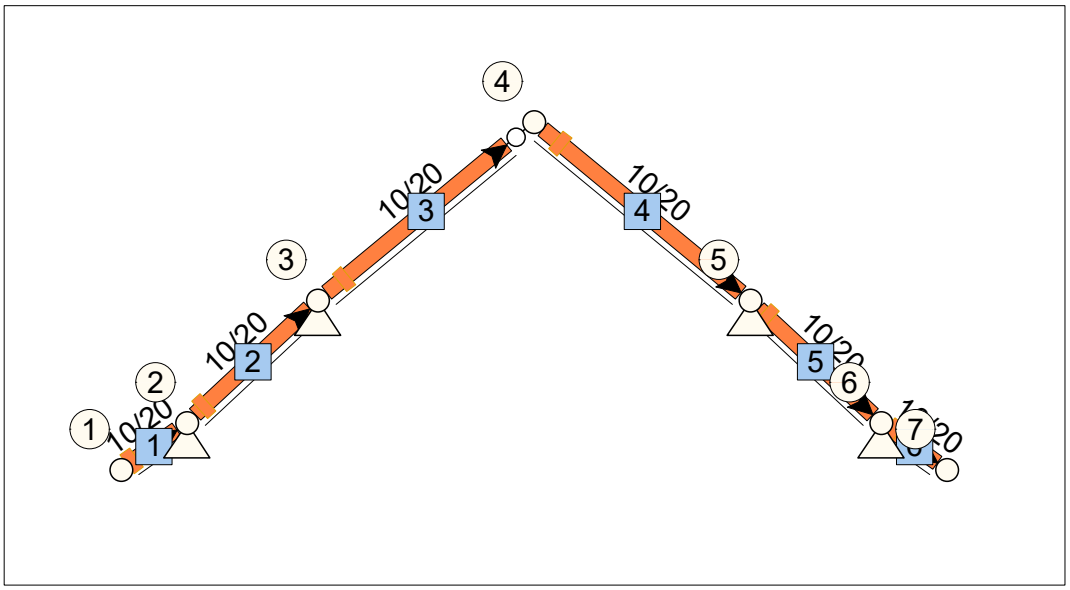


Table of Nodes

Index	X coor. [cm]	Z coor. [cm]	Supports
1:	0.00000	20.000	-
2:	70.000	-30.000	Ux Uz
3:	210.00	-160.00	Ux Uz
4:	440.00	-350.00	-
5:	670.00	-160.00	Ux Uz
6:	810.00	-30.000	Ux Uz
7:	880.00	20.000	-

Table of elements

Index	Type	Node 1	Node 2	Cross-section	Material	Length [cm]	Rel. 1	Rel. 2
1:	Non-linear	1:	2:	1: 10/20	1: C24	86.023	-	-
2:	Non-linear	2:	3:	1: 10/20	1: C24	191.05	-	-
3:	Non-linear	3:	4:	1: 10/20	1: C24	298.33	-	Ry
4:	Non-linear	4:	5:	1: 10/20	1: C24	298.33	-	-
5:	Non-linear	5:	6:	1: 10/20	1: C24	191.05	-	-
6:	Non-linear	6:	7:	1: 10/20	1: C24	86.023	-	-

Table of Mateials

Index	Elastic Modulus [kN/cm2]	Coeff. of Thermal Exp. [1/d°C]
1: C24	1100.0	0.00000

Table of Cross-sections

Index	A [cm2]	Iy [cm4]	Height [cm]
1: 10/20	200.00	6666.7	20.000

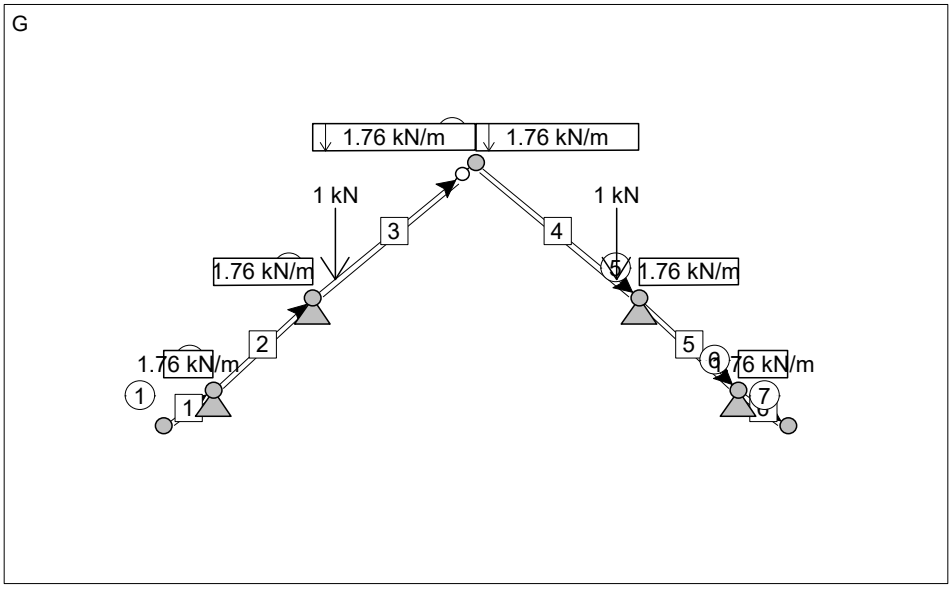
AMSES Frame2D -
Registred to: Statika Fonda

Actions & Load Cases

Load Case	Limit State	G	s	w	s1
G+Q	Ultimate	1.35	1.50	0.90	-
G+Q/ser	Service.	1.00	1.00	0.60	-
Load case: 3	Ultimate	1.35	-	0.90	1.50
Load case: 4	Ultimate	1.00	-	-	-

Load Case	Limit State	G	s	w	s1
Load case: 5	Ultimate	-	1.00	-	-
Load case: 6	Ultimate	-	-	1.00	-
Load case: 7	Ultimate	-	-	-	1.00

A1 : G



A1: G: Uniform Element Loads

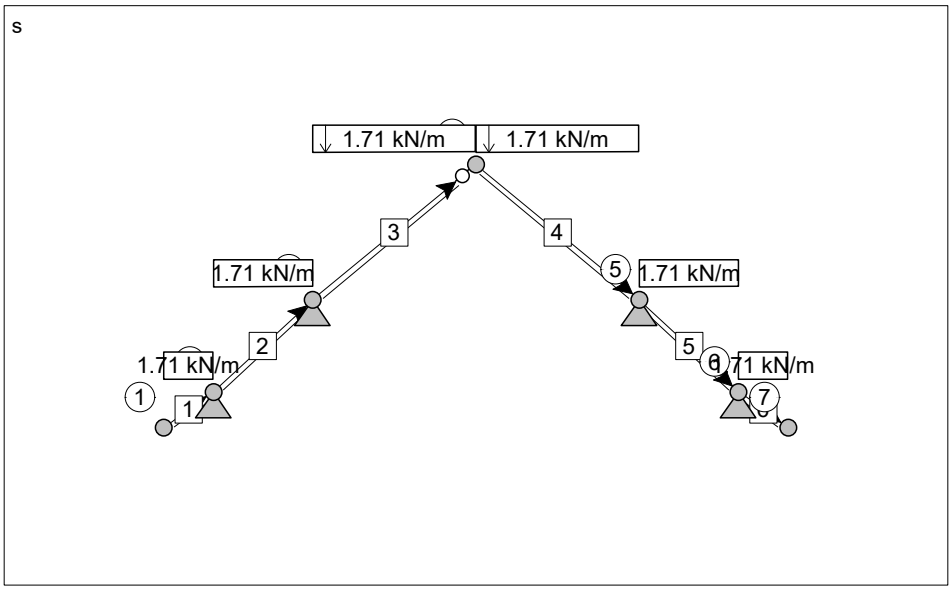
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
4:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
5:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--
6:	0.00000	0.00000	1.7600	1.7600	0.00000	0.00000	World	0.00	1.00	World	--

A1: G: Concentrated Element Loads

Element	Fx [kN]	Fz [kN]	My [kNm]	Direction	Position	Name
3:	0.00000	1.0000	0.00000	World	0.14	--
4:	0.00000	1.0000	0.00000	World	0.86	--

AMSES Frame2D -
Registred to: Statika Fonda

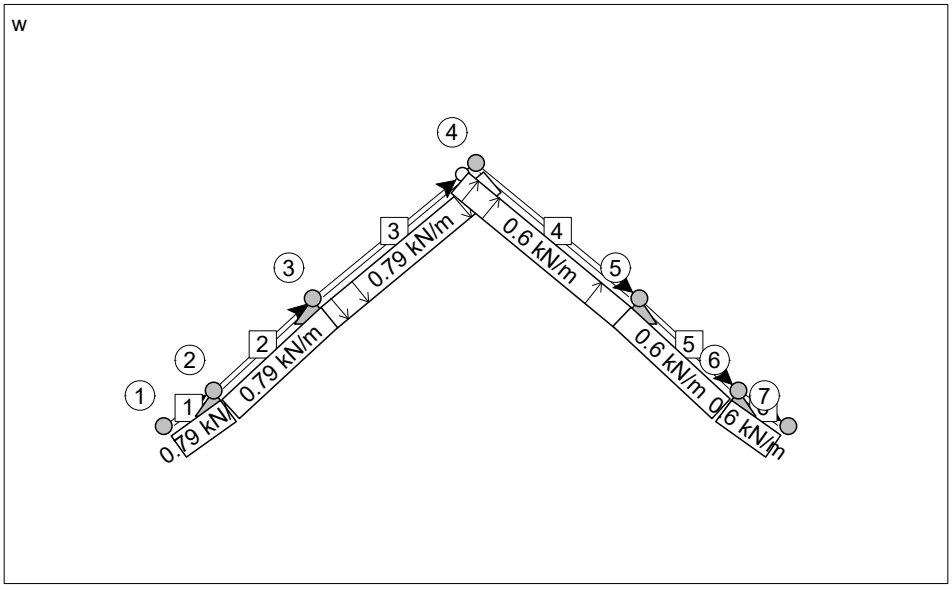
A2 : s



A2: s: Uniform Element Loads

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
4:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
5:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
6:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--

A3 : w



A3: w: Uniform Element Loads

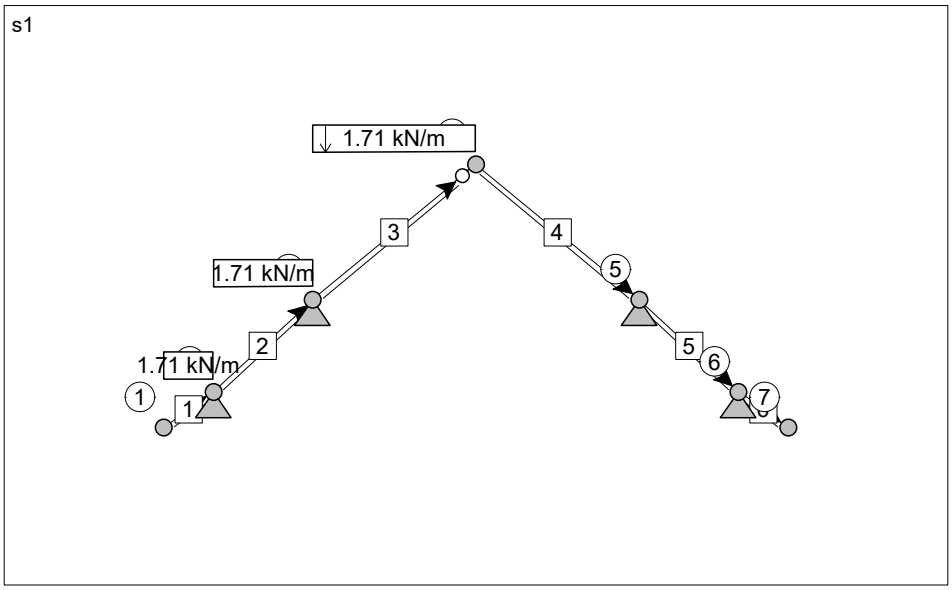
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--
2:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--

AMSES Frame2D -
Registred to: Statika Fonda

A3: w: Uniform Element Loads

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
3:	0.00000	0.00000	0.79000	0.79000	0.00000	0.00000	Local	0.00	1.00	Local	--
4:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--
5:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--
6:	0.00000	0.00000	-0.60000	-0.60000	0.00000	0.00000	Local	0.00	1.00	Local	--

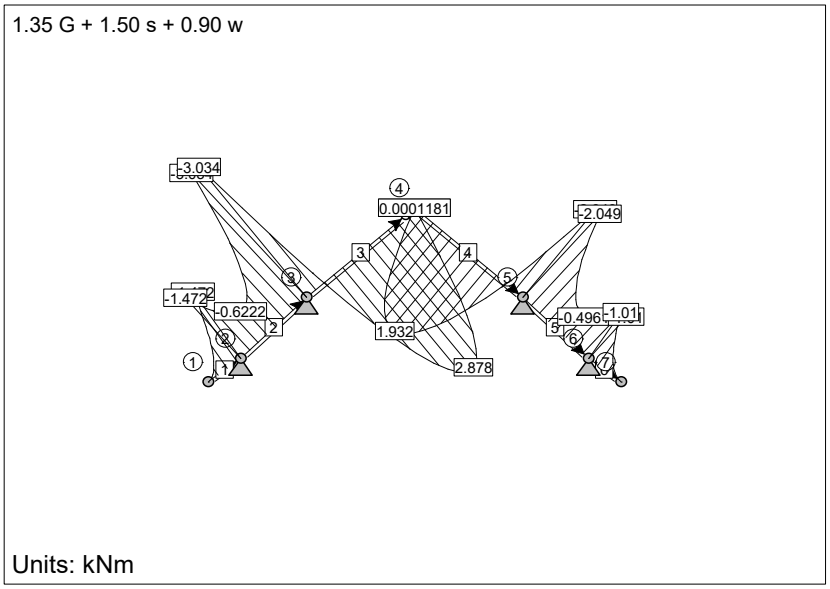
A4 : s1



A4: s1: Uniform Element Loads

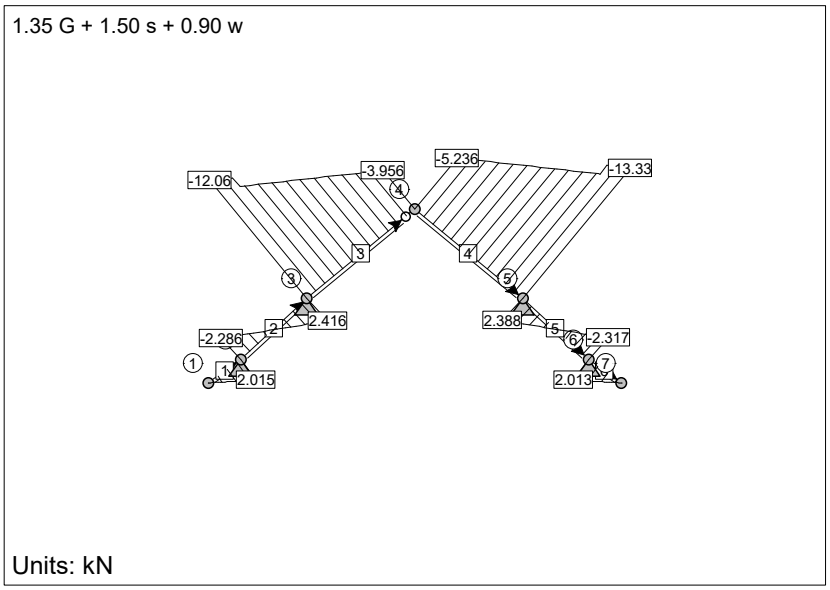
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Direction	Start	End	Position	Name
1:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
2:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--
3:	0.00000	0.00000	1.7100	1.7100	0.00000	0.00000	World	0.00	1.00	World	--

LC1: G+Q: Bending Moments My

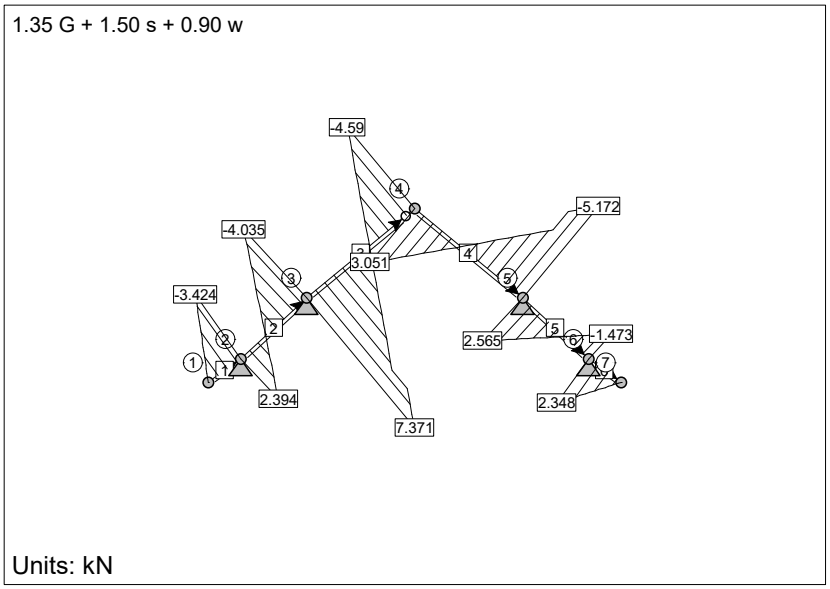


AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Axial Forces Fx



LC1: G+Q: Shear Forces Fz



LC1: G+Q: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.084516	0.11879	0.10534
2:	-	-	0.072364
3:	-	-	-0.11120
4:	0.00082704	0.018725	-0.14283

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.079068
6:	-	-	-0.053238
7:	-0.060997	0.085961	-0.075858

LC1: G+Q: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	-0.31371	-7.2666	-
3:	3.6601	-17.955	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-7.0124	-15.974	-
6:	-0.96270	-5.7376	-

AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.10083	-0.17112	-0.0036800
0.10	0.20166	-0.34224	-0.014720
0.15	0.30249	-0.51336	-0.033120
0.20	0.40332	-0.68448	-0.058881
0.25	0.50415	-0.85560	-0.092002
0.30	0.60497	-1.0267	-0.13248
0.35	0.70579	-1.1979	-0.18032
0.40	0.80660	-1.3690	-0.23553
0.45	0.90740	-1.5401	-0.29809
0.50	1.0082	-1.7113	-0.36801

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	1.1090	-1.8824	-0.44530
0.60	1.2097	-2.0536	-0.52995
0.65	1.3105	-2.2248	-0.62195
0.70	1.4112	-2.3959	-0.72133
0.75	1.5118	-2.5672	-0.82806
0.80	1.6125	-2.7384	-0.94216
0.85	1.7131	-2.9096	-1.0636
0.90	1.8137	-3.0809	-1.1925
0.95	1.9142	-3.2522	-1.3287
1.00	2.0147	-3.4235	-1.4722

LC1: G+Q: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-2.2861	2.3944	-1.4723
0.05	-2.0499	2.0737	-1.2588
0.10	-1.8140	1.7528	-1.0761
0.15	-1.5781	1.4319	-0.92395
0.20	-1.3424	1.1108	-0.80251
0.25	-1.1068	0.78974	-0.71174
0.30	-0.87118	0.46859	-0.65165
0.35	-0.63566	0.14739	-0.62223
0.40	-0.40019	-0.17385	-0.62349
0.45	-0.16477	-0.49512	-0.65544
0.50	0.070584	-0.81644	-0.71808

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.30587	-1.1378	-0.81141
0.60	0.54108	-1.4592	-0.93544
0.65	0.77618	-1.7807	-1.0902
0.70	1.0111	-2.1023	-1.2756
0.75	1.2460	-2.4239	-1.4918
0.80	1.4806	-2.7457	-1.7388
0.85	1.7149	-3.0677	-2.0164
0.90	1.9490	-3.3898	-2.3249
0.95	2.1827	-3.7122	-2.6641
1.00	2.4161	-4.0348	-3.0340

LC1: G+Q: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-12.056	7.3714	-3.0341
0.05	-11.692	6.8325	-1.9747
0.10	-11.330	6.2909	-0.99599
0.14	-11.041	5.8560	-0.27135
0.14	-10.184	4.8126	-0.27137
0.15	-10.112	4.7037	-0.12944
0.20	-9.7516	4.1582	0.53165
0.25	-9.3919	3.6114	1.1111
0.30	-9.0323	3.0638	1.6089
0.35	-8.6727	2.5156	2.0250
0.40	-8.3127	1.9671	2.3593
0.45	-7.9524	1.4183	2.6118

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-7.5916	0.86967	2.7824
0.55	-7.2302	0.32119	2.8713
0.60	-6.8681	-0.22693	2.8783
0.65	-6.5054	-0.77459	2.8037
0.70	-6.1421	-1.3217	2.6473
0.75	-5.7783	-1.8681	2.4094
0.80	-5.4140	-2.4139	2.0900
0.85	-5.0495	-2.9590	1.6892
0.90	-4.6848	-3.5033	1.2072
0.95	-4.3203	-4.0470	0.64415
1.00	-3.9561	-4.5901	0.00017958

AMSES Frame2D -
Registred to: Statika Fonda

LC1: G+Q: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-5.2355	3.0511	0.00011810
0.05	-5.5984	2.6943	0.42848
0.10	-5.9615	2.3369	0.80368
0.15	-6.3246	1.9789	1.1256
0.20	-6.6876	1.6205	1.3941
0.25	-7.0506	1.2615	1.6090
0.30	-7.4133	0.90194	1.7704
0.35	-7.7759	0.54197	1.8781
0.40	-8.1381	0.18159	1.9320
0.45	-8.5001	-0.17912	1.9322
0.50	-8.8618	-0.54006	1.8785
0.55	-9.2232	-0.90113	1.7710

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.60	-9.5844	-1.2622	1.6097
0.65	-9.9454	-1.6232	1.3945
0.70	-10.306	-1.9838	1.1256
0.75	-10.667	-2.3440	0.80281
0.80	-11.028	-2.7036	0.42634
0.85	-11.389	-3.0623	-0.0037917
0.86	-11.461	-3.1339	-0.096204
0.86	-12.319	-4.1765	-0.096195
0.90	-12.608	-4.4623	-0.61155
0.95	-12.970	-4.8180	-1.3037
1.00	-13.333	-5.1716	-2.0487

LC1: G+Q: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	2.3879	2.5650	-2.0487
0.05	2.1534	2.3623	-1.8134
0.10	1.9188	2.1597	-1.5974
0.15	1.6840	1.9573	-1.4008
0.20	1.4491	1.7550	-1.2234
0.25	1.2141	1.5529	-1.0654
0.30	0.97899	1.3508	-0.92675
0.35	0.74382	1.1488	-0.80737
0.40	0.50860	0.94680	-0.70728
0.45	0.27332	0.74490	-0.62648
0.50	0.038012	0.54303	-0.56497

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.19733	0.34121	-0.52274
0.60	-0.43271	0.13941	-0.49979
0.65	-0.66811	-0.062355	-0.49611
0.70	-0.90354	-0.26409	-0.51170
0.75	-1.1390	-0.46579	-0.54656
0.80	-1.3745	-0.66745	-0.60068
0.85	-1.6100	-0.86906	-0.67406
0.90	-1.8456	-1.0706	-0.76670
0.95	-2.0812	-1.2721	-0.87861
1.00	-2.3169	-1.4735	-1.0098

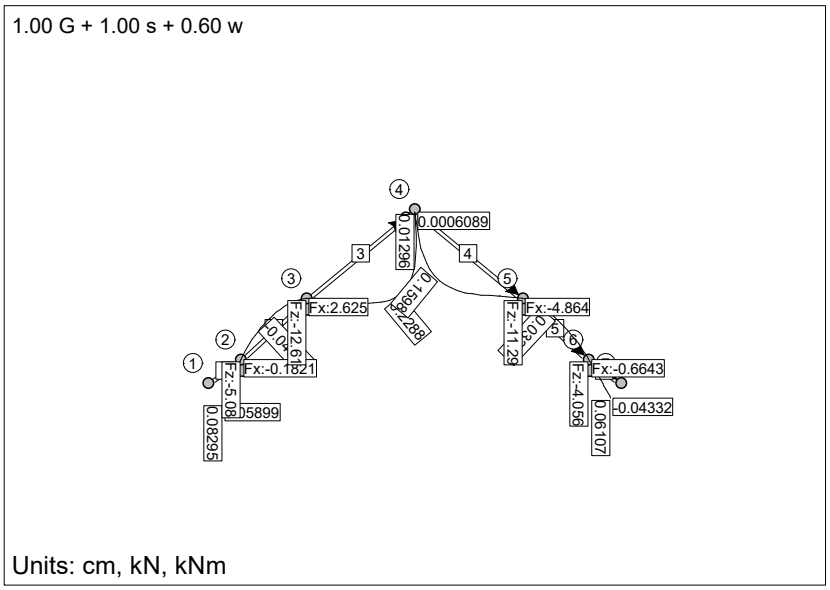
LC1: G+Q: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	2.0125	2.3481	-1.0097
0.05	1.9120	2.2306	-0.91127
0.10	1.8115	2.1131	-0.81786
0.15	1.7109	1.9956	-0.72950
0.20	1.6104	1.8781	-0.64619
0.25	1.5098	1.7607	-0.56793
0.30	1.4092	1.6433	-0.49473
0.35	1.3086	1.5259	-0.42657
0.40	1.2079	1.4085	-0.36347
0.45	1.1073	1.2911	-0.30541
0.50	1.0067	1.1737	-0.25240

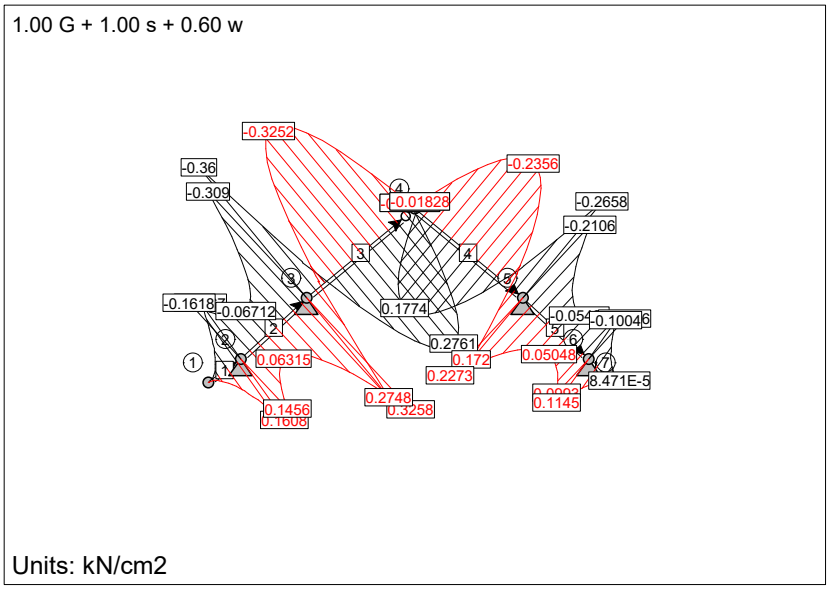
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.90601	1.0563	-0.20445
0.60	0.80535	0.93893	-0.16154
0.65	0.70469	0.82156	-0.12368
0.70	0.60402	0.70419	-0.090865
0.75	0.50336	0.58682	-0.063100
0.80	0.40269	0.46946	-0.040384
0.85	0.30202	0.35209	-0.022716
0.90	0.20134	0.23473	-0.010096
0.95	0.10067	0.11736	-0.0025240
1.00	0.00000	0.00000	0.00000

AMSES Frame2D -
Registred to: Statika Fonda

LC2: G+Q/ser: Displacements and Reactions



LC2: G+Q/ser: Stress



LC2: G+Q/ser: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.058985	0.082950	0.073533
2:	-	-	0.050574
3:	-	-	-0.077706
4:	0.00060891	0.012957	-0.10156

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.056307
6:	-	-	-0.037830
7:	-0.043320	0.061067	-0.053881

LC2: G+Q/ser: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	-0.18211	-5.0800	-
3:	2.6246	-12.609	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-4.8640	-11.292	-
6:	-0.66429	-4.0562	-

AMSES Frame2D -
Registred to: Statika Fonda

LC2: G+Q/ser: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.070744	-0.11912	-0.0025619
0.10	0.14149	-0.23825	-0.010247
0.15	0.21223	-0.35737	-0.023057
0.20	0.28298	-0.47650	-0.040990
0.25	0.35372	-0.59563	-0.064047
0.30	0.42446	-0.71475	-0.092228
0.35	0.49520	-0.83388	-0.12553
0.40	0.56593	-0.95301	-0.16396
0.45	0.63666	-1.0721	-0.20751
0.50	0.70738	-1.1913	-0.25619

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.77810	-1.3104	-0.30999
0.60	0.84881	-1.4296	-0.36892
0.65	0.91951	-1.5487	-0.43297
0.70	0.99019	-1.6679	-0.50215
0.75	1.0609	-1.7871	-0.57645
0.80	1.1315	-1.9062	-0.65587
0.85	1.2022	-2.0254	-0.74043
0.90	1.2728	-2.1446	-0.83011
0.95	1.3434	-2.2638	-0.92492
1.00	1.4139	-2.3831	-1.0249

LC2: G+Q/ser: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.6198	1.6607	-1.0249
0.05	-1.4542	1.4378	-0.87686
0.10	-1.2886	1.2148	-0.75016
0.15	-1.1231	0.99171	-0.64477
0.20	-0.95759	0.76860	-0.56070
0.25	-0.79217	0.54546	-0.49794
0.30	-0.62677	0.32230	-0.45650
0.35	-0.46140	0.099109	-0.43637
0.40	-0.29606	-0.12410	-0.43757
0.45	-0.13075	-0.34733	-0.46008
0.50	0.034537	-0.57057	-0.50392

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.19979	-0.79384	-0.56908
0.60	0.36500	-1.0171	-0.65558
0.65	0.53016	-1.2405	-0.76340
0.70	0.69526	-1.4638	-0.89257
0.75	0.86028	-1.6873	-1.0431
0.80	1.0252	-1.9107	-1.2149
0.85	1.1900	-2.1343	-1.4081
0.90	1.3547	-2.3580	-1.6227
0.95	1.5192	-2.5817	-1.8586
1.00	1.6835	-2.8056	-2.1159

LC2: G+Q/ser: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-8.5247	5.1648	-2.1159
0.05	-8.2694	4.7890	-1.3736
0.10	-8.0150	4.4119	-0.68736
0.14	-7.8120	4.1094	-0.17901
0.14	-7.1766	3.3371	-0.17899
0.15	-7.1259	3.2614	-0.080585
0.20	-6.8726	2.8823	0.37770
0.25	-6.6195	2.5027	0.77933
0.30	-6.3665	2.1226	1.1243
0.35	-6.1134	1.7422	1.4125
0.40	-5.8602	1.3617	1.6440
0.45	-5.6068	0.98110	1.8187

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-5.3532	0.60051	1.9366
0.55	-5.0992	0.22002	1.9979
0.60	-4.8450	-0.16030	2.0023
0.65	-4.5905	-0.54039	1.9501
0.70	-4.3356	-0.92020	1.8411
0.75	-4.0806	-1.2997	1.6756
0.80	-3.8253	-1.6789	1.4534
0.85	-3.5698	-2.0577	1.1747
0.90	-3.3143	-2.4362	0.83953
0.95	-3.0589	-2.8143	0.44796
1.00	-2.8037	-3.1922	8.7385E-05

AMSES Frame2D -
Registred to: Statika Fonda

LC2: G+Q/ser: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-3.6546	2.1666	5.8689E-05
0.05	-3.9092	1.9130	0.30426
0.10	-4.1640	1.6591	0.57066
0.15	-4.4187	1.4050	0.79920
0.20	-4.6735	1.1506	0.98981
0.25	-4.9282	0.89588	1.1425
0.30	-5.1828	0.64094	1.2571
0.35	-5.4372	0.38577	1.3337
0.40	-5.6916	0.13040	1.3721
0.45	-5.9458	-0.12513	1.3725
0.50	-6.1998	-0.38079	1.3348
0.55	-6.4537	-0.63651	1.2589
0.60	-6.7075	-0.89223	1.1449
0.65	-6.9612	-1.1479	0.99276
0.70	-7.2149	-1.4034	0.80250
0.75	-7.4685	-1.6587	0.57413
0.80	-7.7222	-1.9137	0.30769
0.85	-7.9759	-2.1682	0.0032065
0.86	-8.0266	-2.2191	-0.062220
0.86	-8.6625	-2.9910	-0.062239
0.90	-8.8656	-3.1940	-0.43122
0.95	-9.1199	-3.4471	-0.92652
1.00	-9.3746	-3.6991	-1.4595

LC2: G+Q/ser: Element 5:

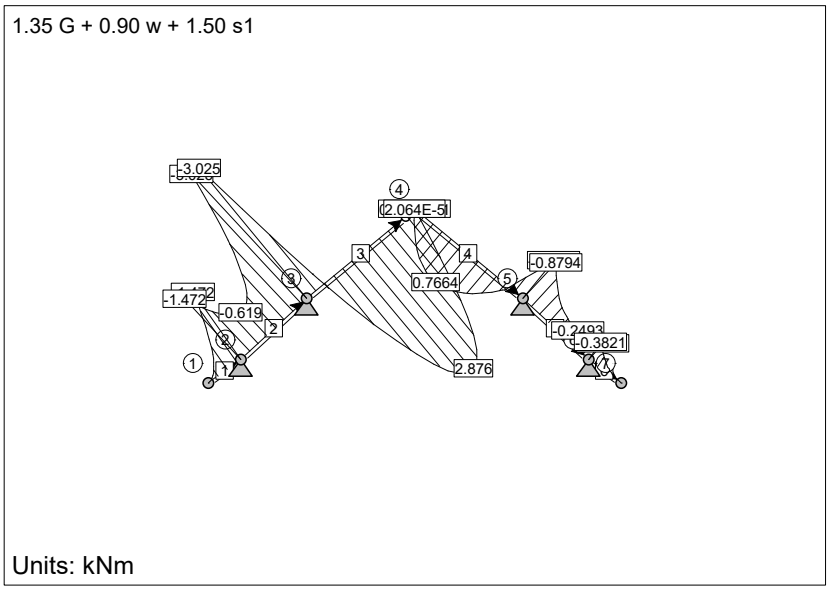
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	1.6702	1.8262	-1.4595
0.05	1.5054	1.6822	-1.2919
0.10	1.3405	1.5382	-1.1381
0.15	1.1755	1.3944	-0.99803
0.20	1.0104	1.2506	-0.87170
0.25	0.84532	1.1068	-0.75910
0.30	0.68017	0.96308	-0.66024
0.35	0.51498	0.81941	-0.57510
0.40	0.34976	0.67576	-0.50369
0.45	0.18451	0.53213	-0.44600
0.50	0.019251	0.38853	-0.40203
0.55	-0.14603	0.24494	-0.37177
0.60	-0.31133	0.10137	-0.35523
0.65	-0.47663	-0.042182	-0.35241
0.70	-0.64196	-0.18572	-0.36329
0.75	-0.80729	-0.32924	-0.38789
0.80	-0.97265	-0.47275	-0.42619
0.85	-1.1380	-0.61622	-0.47820
0.90	-1.3034	-0.75967	-0.54391
0.95	-1.4688	-0.90309	-0.62333
1.00	-1.6343	-1.0465	-0.71646

LC2: G+Q/ser: Element 6:

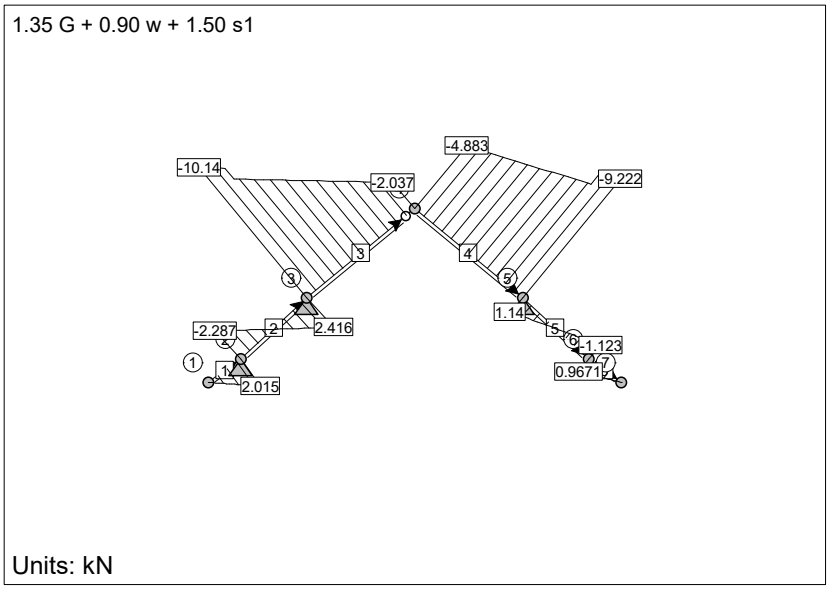
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	1.4129	1.6659	-0.71645
0.05	1.3423	1.5826	-0.64659
0.10	1.2717	1.4993	-0.58031
0.15	1.2011	1.4159	-0.51762
0.20	1.1305	1.3326	-0.45851
0.25	1.0599	1.2493	-0.40298
0.30	0.98926	1.1660	-0.35104
0.35	0.91862	1.0827	-0.30268
0.40	0.84798	0.99938	-0.25790
0.45	0.77732	0.91609	-0.21671
0.50	0.70667	0.83280	-0.17910
0.55	0.63601	0.74951	-0.14507
0.60	0.56535	0.66623	-0.11462
0.65	0.49468	0.58295	-0.087757
0.70	0.42401	0.49967	-0.064474
0.75	0.35335	0.41639	-0.044774
0.80	0.28268	0.33311	-0.028655
0.85	0.21201	0.24983	-0.016118
0.90	0.14134	0.16655	-0.0071637
0.95	0.070670	0.083277	-0.0017909
1.00	0.00000	0.00000	0.00000

AMSES Frame2D -
Registred to: Statika Fonda

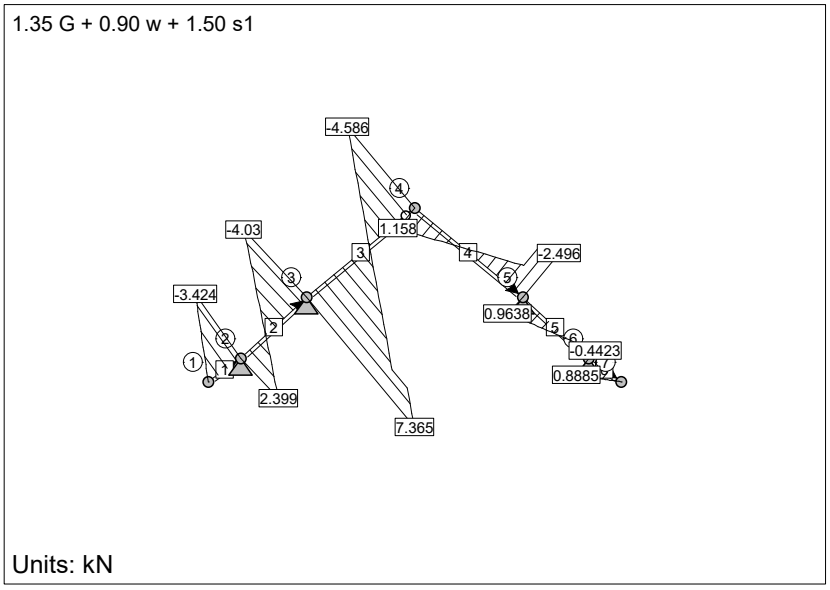
LC3: Load case: 3: Bending Moments My



LC3: Load case: 3: Axial Forces Fx



LC3: Load case: 3: Shear Forces Fz



AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.084316	0.11851	0.10512
2:	-	-	0.072135
3:	-	-	-0.11074
4:	0.00034036	0.014049	-0.056182

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.036556
6:	-	-	-0.024186
7:	-0.026566	0.037496	-0.032747

LC3: Load case: 3: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	-0.31654	-7.2705	-
3:	2.1853	-16.727	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-5.7058	-9.2759	-
6:	-0.79173	-2.3739	-

LC3: Load case: 3: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.10083	-0.17112	-0.0036800
0.10	0.20166	-0.34224	-0.014720
0.15	0.30249	-0.51336	-0.033120
0.20	0.40332	-0.68448	-0.058881
0.25	0.50414	-0.85560	-0.092002
0.30	0.60497	-1.0267	-0.13248
0.35	0.70578	-1.1979	-0.18032
0.40	0.80659	-1.3690	-0.23553
0.45	0.90739	-1.5401	-0.29809
0.50	1.0082	-1.7113	-0.36801

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	1.1090	-1.8824	-0.44530
0.60	1.2097	-2.0536	-0.52995
0.65	1.3104	-2.2248	-0.62196
0.70	1.4112	-2.3960	-0.72133
0.75	1.5118	-2.5672	-0.82807
0.80	1.6125	-2.7384	-0.94217
0.85	1.7131	-2.9096	-1.0636
0.90	1.8137	-3.0809	-1.1925
0.95	1.9142	-3.2522	-1.3287
1.00	2.0146	-3.4236	-1.4722

LC3: Load case: 3: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-2.2867	2.3992	-1.4723
0.05	-2.0506	2.0785	-1.2584
0.10	-1.8146	1.7576	-1.0751
0.15	-1.5788	1.4367	-0.92257
0.20	-1.3430	1.1157	-0.80067
0.25	-1.1074	0.79456	-0.70944
0.30	-0.87180	0.47341	-0.64889
0.35	-0.63628	0.15221	-0.61901
0.40	-0.40081	-0.16903	-0.61981
0.45	-0.16540	-0.49030	-0.65130
0.50	0.069961	-0.81162	-0.71347

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.30525	-1.1330	-0.80634
0.60	0.54046	-1.4544	-0.92992
0.65	0.77556	-1.7759	-1.0842
0.70	1.0105	-2.0974	-1.2692
0.75	1.2453	-2.4191	-1.4849
0.80	1.4800	-2.7409	-1.7314
0.85	1.7143	-3.0629	-2.0086
0.90	1.9484	-3.3850	-2.3166
0.95	2.1822	-3.7074	-2.6553
1.00	2.4155	-4.0300	-3.0248

AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-10.137	7.3647	-3.0249
0.05	-9.7731	6.8248	-1.9666
0.10	-9.4107	6.2826	-0.98905
0.14	-9.1218	5.8475	-0.26541
0.14	-8.2649	4.8041	-0.26543
0.15	-8.1928	4.6952	-0.12375
0.20	-7.8327	4.1497	0.53608
0.25	-7.4729	3.6033	1.1143
0.30	-7.1133	3.0563	1.6110
0.35	-6.7537	2.5088	2.0260
0.40	-6.3937	1.9611	2.3593
0.45	-6.0334	1.4133	2.6109

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-5.6726	0.86570	2.7809
0.55	-5.3111	0.31834	2.8693
0.60	-4.9491	-0.22865	2.8760
0.65	-4.5863	-0.77519	2.8011
0.70	-4.2230	-1.3212	2.6448
0.75	-3.8592	-1.8667	2.4070
0.80	-3.4950	-2.4115	2.0879
0.85	-3.1304	-2.9559	1.6875
0.90	-2.7658	-3.4997	1.2060
0.95	-2.4012	-4.0430	0.64354
1.00	-2.0370	-4.5859	0.00017907

LC3: Load case: 3: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.8826	1.1578	2.0638E-05
0.05	-5.0568	1.0277	0.16299
0.10	-5.2310	0.89755	0.30658
0.15	-5.4052	0.76719	0.43074
0.20	-5.5794	0.63668	0.53545
0.25	-5.7535	0.50602	0.62068
0.30	-5.9277	0.37523	0.68641
0.35	-6.1018	0.24432	0.73261
0.40	-6.2759	0.11332	0.75928
0.45	-6.4499	-0.017759	0.76641
0.50	-6.6239	-0.14889	0.75397
0.55	-6.7979	-0.28004	0.72198

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.60	-6.9718	-0.41118	0.67043
0.65	-7.1457	-0.54229	0.59933
0.70	-7.3196	-0.67332	0.50867
0.75	-7.4935	-0.80424	0.39848
0.80	-7.6673	-0.93500	0.26876
0.85	-7.8412	-1.0656	0.11954
0.86	-7.8760	-1.0916	0.087373
0.86	-8.7349	-2.1332	0.087345
0.90	-8.8740	-2.2373	-0.17339
0.95	-9.0481	-2.3669	-0.51678
1.00	-9.2224	-2.4959	-0.87945

LC3: Load case: 3: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	1.1397	0.96382	-0.87945
0.05	1.0267	0.89334	-0.79075
0.10	0.91361	0.82289	-0.70878
0.15	0.80053	0.75248	-0.63353
0.20	0.68743	0.68209	-0.56501
0.25	0.57432	0.61173	-0.50322
0.30	0.46120	0.54138	-0.44814
0.35	0.34806	0.47105	-0.39978
0.40	0.23491	0.40074	-0.35815
0.45	0.12176	0.33044	-0.32322
0.50	0.0085948	0.26014	-0.29502

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.10457	0.18986	-0.27353
0.60	-0.21775	0.11958	-0.25875
0.65	-0.33093	0.049310	-0.25068
0.70	-0.44411	-0.020952	-0.24933
0.75	-0.55730	-0.091207	-0.25468
0.80	-0.67050	-0.16145	-0.26675
0.85	-0.78370	-0.23169	-0.28553
0.90	-0.89690	-0.30192	-0.31101
0.95	-1.0101	-0.37213	-0.34321
1.00	-1.1233	-0.44233	-0.38211

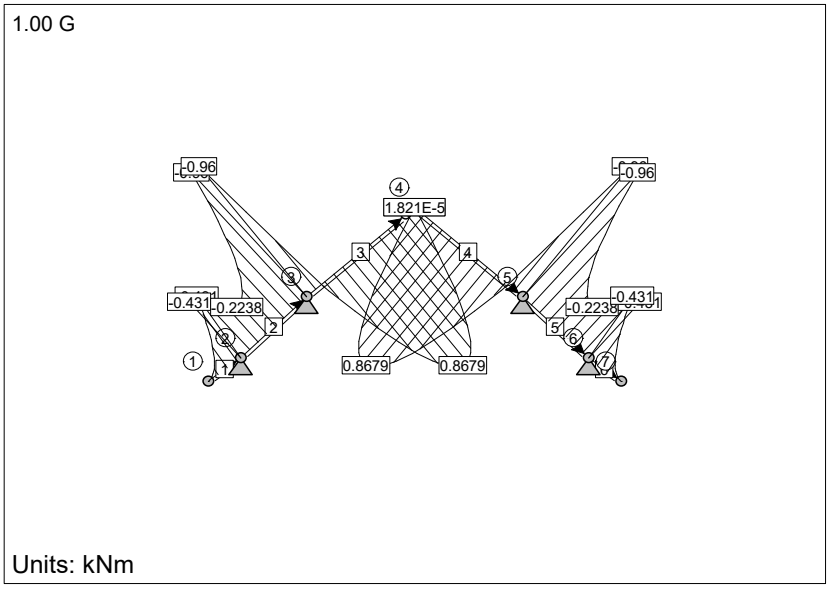
AMSES Frame2D -
Registred to: Statika Fonda

LC3: Load case: 3: Element 6:

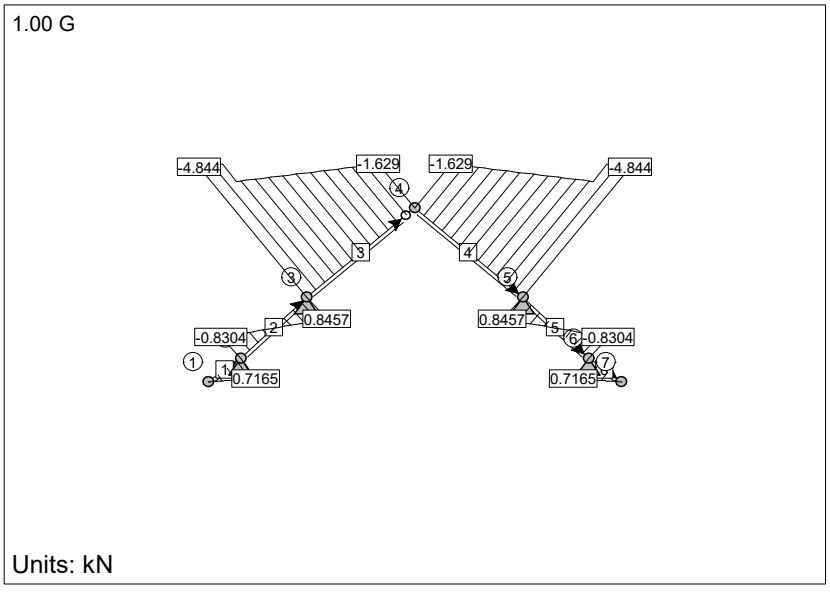
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.96709	0.88847	-0.38211
0.05	0.91875	0.84402	-0.34485
0.10	0.87041	0.79959	-0.30950
0.15	0.82207	0.75515	-0.27607
0.20	0.77372	0.71072	-0.24454
0.25	0.72538	0.66629	-0.21493
0.30	0.67702	0.62186	-0.18722
0.35	0.62867	0.57744	-0.16143
0.40	0.58032	0.53301	-0.13755
0.45	0.53196	0.48859	-0.11558
0.50	0.48360	0.44417	-0.095521

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.43525	0.39975	-0.077372
0.60	0.38689	0.35533	-0.061134
0.65	0.33853	0.31092	-0.046805
0.70	0.29017	0.26650	-0.034388
0.75	0.24181	0.22208	-0.023880
0.80	0.19344	0.17766	-0.015283
0.85	0.14508	0.13325	-0.0085968
0.90	0.096722	0.088832	-0.0038208
0.95	0.048361	0.044416	-0.00095520
1.00	0.00000	0.00000	0.00000

LC4: Load case: 4: Bending Moments My

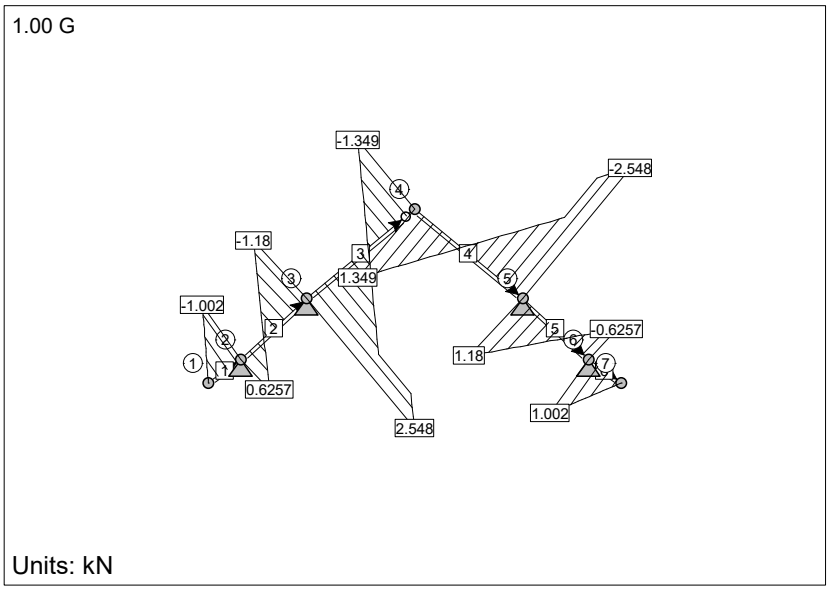


LC4: Load case: 4: Axial Forces Fx



AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Shear Forces Fz



LC4: Load case: 4: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.027051	0.038091	0.033531
2:	-	-	0.023875
3:	-	-	-0.037031
4:	0.00000	0.0065312	-0.064634

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.037031
6:	-	-	-0.023875
7:	-0.027051	0.038091	-0.033531

LC4: Load case: 4: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	0.18232	-2.2557	-
3:	1.9330	-6.4883	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-1.9330	-6.4883	-
6:	-0.18232	-2.2557	-

LC4: Load case: 4: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.035834	-0.050105	-0.0010775
0.10	0.071667	-0.10021	-0.0043102
0.15	0.10750	-0.15032	-0.0096979
0.20	0.14333	-0.20042	-0.017241
0.25	0.17917	-0.25053	-0.026939
0.30	0.21500	-0.30063	-0.038792
0.35	0.25083	-0.35074	-0.052800
0.40	0.28666	-0.40084	-0.068963
0.45	0.32250	-0.45095	-0.087282
0.50	0.35833	-0.50106	-0.10776

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.39415	-0.55117	-0.13038
0.60	0.42998	-0.60128	-0.15517
0.65	0.46581	-0.65139	-0.18211
0.70	0.50163	-0.70150	-0.21120
0.75	0.53745	-0.75161	-0.24245
0.80	0.57327	-0.80173	-0.27586
0.85	0.60908	-0.85185	-0.31142
0.90	0.64489	-0.90197	-0.34914
0.95	0.68070	-0.95209	-0.38901
1.00	0.71650	-1.0022	-0.43104

AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.83042	0.62573	-0.43105
0.05	-0.74652	0.53552	-0.37558
0.10	-0.66263	0.44530	-0.32873
0.15	-0.57876	0.35507	-0.29050
0.20	-0.49489	0.26484	-0.26089
0.25	-0.41103	0.17459	-0.23991
0.30	-0.32718	0.084336	-0.22754
0.35	-0.24333	-0.0059215	-0.22380
0.40	-0.15949	-0.096184	-0.22867
0.45	-0.075655	-0.18645	-0.24217
0.50	0.0081760	-0.27672	-0.26429

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.091999	-0.36700	-0.29504
0.60	0.17581	-0.45729	-0.33441
0.65	0.25962	-0.54758	-0.38240
0.70	0.34341	-0.63789	-0.43902
0.75	0.42719	-0.72820	-0.50427
0.80	0.51094	-0.81853	-0.57815
0.85	0.59468	-0.90888	-0.66066
0.90	0.67839	-0.99925	-0.75179
0.95	0.76207	-1.0896	-0.85156
1.00	0.84571	-1.1801	-0.95997

LC4: Load case: 4: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.8442	2.5477	-0.95997
0.05	-4.7150	2.3923	-0.59153
0.10	-4.5860	2.2366	-0.24630
0.14	-4.4830	2.1118	0.013110
0.14	-3.8468	1.3402	0.013143
0.15	-3.8210	1.3090	0.052644
0.20	-3.6923	1.1527	0.23626
0.25	-3.5636	0.99631	0.39653
0.30	-3.4349	0.83983	0.53347
0.35	-3.3062	0.68330	0.64707
0.40	-3.1775	0.52673	0.73731
0.45	-3.0487	0.37015	0.80420

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-2.9199	0.21358	0.84773
0.55	-2.7910	0.057033	0.86792
0.60	-2.6621	-0.099474	0.86476
0.65	-2.5331	-0.25593	0.83825
0.70	-2.4040	-0.41232	0.78841
0.75	-2.2750	-0.56864	0.71525
0.80	-2.1459	-0.72488	0.61877
0.85	-2.0167	-0.88104	0.49899
0.90	-1.8876	-1.0371	0.35593
0.95	-1.7584	-1.1931	0.18960
1.00	-1.6293	-1.3491	1.8212E-05

LC4: Load case: 4: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.6293	1.3491	1.8212E-05
0.05	-1.7584	1.1931	0.18960
0.10	-1.8876	1.0371	0.35593
0.15	-2.0167	0.88104	0.49899
0.20	-2.1459	0.72488	0.61877
0.25	-2.2750	0.56864	0.71525
0.30	-2.4040	0.41232	0.78841
0.35	-2.5331	0.25593	0.83825
0.40	-2.6621	0.099474	0.86476
0.45	-2.7910	-0.057033	0.86792
0.50	-2.9199	-0.21358	0.84773
0.55	-3.0487	-0.37015	0.80420

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.60	-3.1775	-0.52673	0.73731
0.65	-3.3062	-0.68330	0.64707
0.70	-3.4349	-0.83983	0.53347
0.75	-3.5636	-0.99631	0.39653
0.80	-3.6923	-1.1527	0.23626
0.85	-3.8210	-1.3090	0.052644
0.86	-3.8468	-1.3402	0.013143
0.86	-4.4830	-2.1118	0.013110
0.90	-4.5860	-2.2366	-0.24630
0.95	-4.7150	-2.3923	-0.59153
1.00	-4.8442	-2.5477	-0.95997

AMSES Frame2D -
Registred to: Statika Fonda

LC4: Load case: 4: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.84571	1.1801	-0.95997
0.05	0.76207	1.0896	-0.85156
0.10	0.67839	0.99925	-0.75179
0.15	0.59468	0.90888	-0.66066
0.20	0.51094	0.81853	-0.57815
0.25	0.42719	0.72820	-0.50427
0.30	0.34341	0.63789	-0.43902
0.35	0.25962	0.54758	-0.38240
0.40	0.17581	0.45729	-0.33441
0.45	0.091999	0.36700	-0.29504
0.50	0.0081760	0.27672	-0.26429

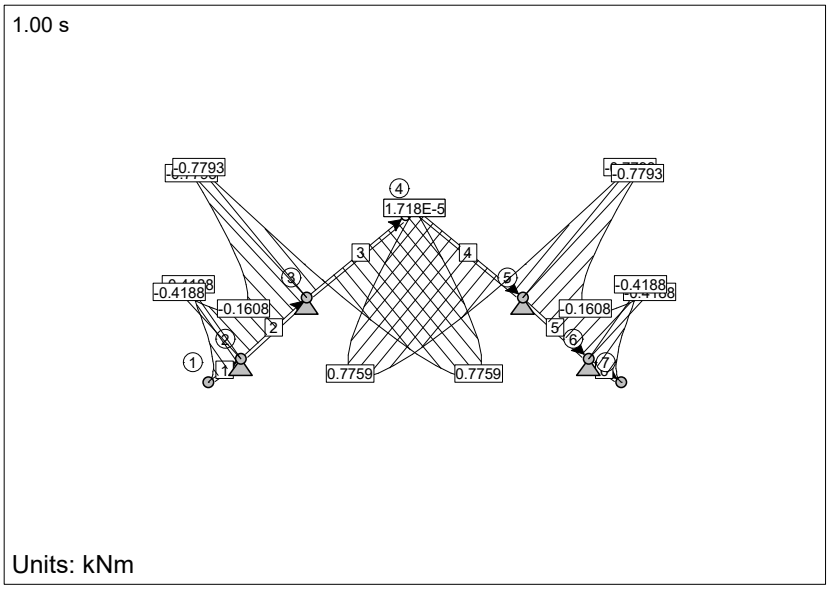
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	-0.075655	0.18645	-0.24217
0.60	-0.15949	0.096184	-0.22867
0.65	-0.24333	0.0059215	-0.22380
0.70	-0.32718	-0.084336	-0.22754
0.75	-0.41103	-0.17459	-0.23991
0.80	-0.49489	-0.26484	-0.26089
0.85	-0.57876	-0.35507	-0.29050
0.90	-0.66263	-0.44530	-0.32873
0.95	-0.74652	-0.53552	-0.37558
1.00	-0.83042	-0.62573	-0.43105

LC4: Load case: 4: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.71650	1.0022	-0.43104
0.05	0.68070	0.95209	-0.38901
0.10	0.64489	0.90197	-0.34914
0.15	0.60908	0.85185	-0.31142
0.20	0.57327	0.80173	-0.27586
0.25	0.53745	0.75161	-0.24245
0.30	0.50163	0.70150	-0.21120
0.35	0.46581	0.65139	-0.18211
0.40	0.42998	0.60128	-0.15517
0.45	0.39415	0.55117	-0.13038
0.50	0.35833	0.50106	-0.10776

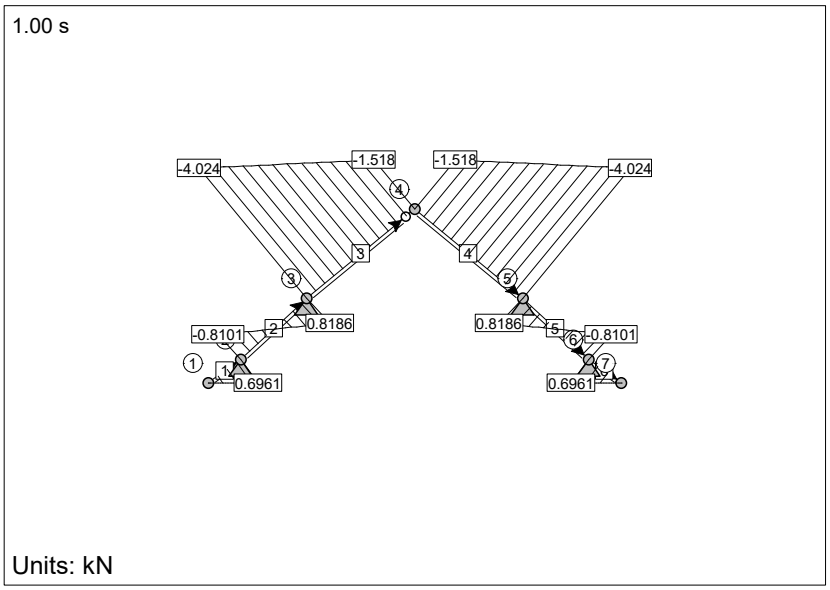
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.32250	0.45095	-0.087282
0.60	0.28666	0.40084	-0.068963
0.65	0.25083	0.35074	-0.052800
0.70	0.21500	0.30063	-0.038792
0.75	0.17917	0.25053	-0.026939
0.80	0.14333	0.20042	-0.017241
0.85	0.10750	0.15032	-0.0096979
0.90	0.071667	0.10021	-0.0043102
0.95	0.035834	0.050105	-0.0010775
1.00	0.00000	0.00000	0.00000

LC5: Load case: 5: Bending Moments My

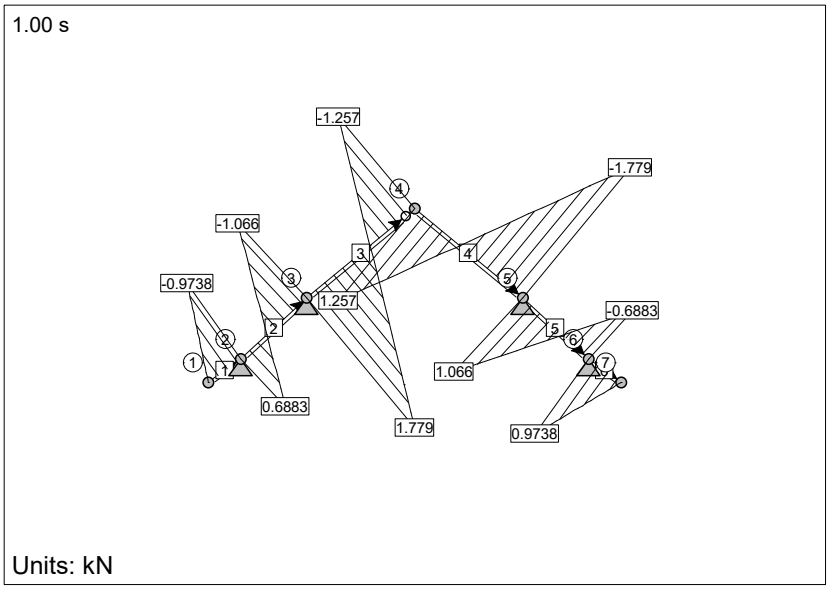


AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Axial Forces Fx



LC5: Load case: 5: Shear Forces Fz



LC5: Load case: 5: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.022952	0.032352	0.028765
2:	-	-	0.019383
3:	-	-	-0.028350
4:	0.00000	0.0059937	-0.056851

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.028350
6:	-	-	-0.019383
7:	-0.022952	0.032352	-0.028765

LC5: Load case: 5: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	0.12493	-2.2526	-
3:	1.8461	-5.2714	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-1.8461	-5.2714	-
6:	-0.12493	-2.2526	-

AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.034812	-0.048684	-0.0010470
0.10	0.069623	-0.097369	-0.0041880
0.15	0.10443	-0.14605	-0.0094230
0.20	0.13925	-0.19474	-0.016752
0.25	0.17406	-0.24342	-0.026175
0.30	0.20887	-0.29211	-0.037692
0.35	0.24368	-0.34079	-0.051303
0.40	0.27849	-0.38948	-0.067008
0.45	0.31330	-0.43816	-0.084807
0.50	0.34811	-0.48685	-0.10470

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.38291	-0.53554	-0.12669
0.60	0.41772	-0.58423	-0.15077
0.65	0.45252	-0.63292	-0.17695
0.70	0.48732	-0.68161	-0.20522
0.75	0.52212	-0.73030	-0.23558
0.80	0.55692	-0.77900	-0.26804
0.85	0.59171	-0.82770	-0.30259
0.90	0.62650	-0.87640	-0.33924
0.95	0.66129	-0.92510	-0.37798
1.00	0.69607	-0.97380	-0.41882

LC5: Load case: 5: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.81008	0.68827	-0.41882
0.05	-0.72857	0.60061	-0.35726
0.10	-0.64707	0.51295	-0.30407
0.15	-0.56559	0.42528	-0.25926
0.20	-0.48411	0.33759	-0.22282
0.25	-0.40264	0.24990	-0.19477
0.30	-0.32117	0.16221	-0.17508
0.35	-0.23971	0.074507	-0.16378
0.40	-0.15825	-0.013197	-0.16085
0.45	-0.076792	-0.10090	-0.16630
0.50	0.0046592	-0.18861	-0.18013

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.086106	-0.27633	-0.20233
0.60	0.16755	-0.36405	-0.23292
0.65	0.24898	-0.45177	-0.27188
0.70	0.33041	-0.53950	-0.31923
0.75	0.41182	-0.62724	-0.37496
0.80	0.49322	-0.71500	-0.43907
0.85	0.57461	-0.80276	-0.51156
0.90	0.65597	-0.89055	-0.59243
0.95	0.73731	-0.97835	-0.68170
1.00	0.81862	-1.0662	-0.77934

LC5: Load case: 5: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-4.0236	1.7788	-0.77935
0.05	-3.8982	1.6277	-0.52528
0.10	-3.7729	1.4763	-0.29378
0.14	-3.6728	1.3551	-0.12486
0.14	-3.6728	1.3551	-0.12484
0.15	-3.6478	1.3248	-0.084882
0.20	-3.5226	1.1730	0.10142
0.25	-3.3975	1.0212	0.26508
0.30	-3.2725	0.86929	0.40607
0.35	-3.1474	0.71731	0.52440
0.40	-3.0223	0.56528	0.62005
0.45	-2.8972	0.41322	0.69303

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-2.7720	0.26117	0.74333
0.55	-2.6468	0.10913	0.77095
0.60	-2.5215	-0.042872	0.77589
0.65	-2.3963	-0.19484	0.75817
0.70	-2.2709	-0.34675	0.71778
0.75	-2.1455	-0.49860	0.65473
0.80	-2.0201	-0.65038	0.56903
0.85	-1.8947	-0.80210	0.46069
0.90	-1.7692	-0.95375	0.32974
0.95	-1.6438	-1.1053	0.17617
1.00	-1.5184	-1.2568	1.7176E-05

AMSES Frame2D -
Registred to: Statika Fonda

LC5: Load case: 5: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.5184	1.2568	1.7176E-05
0.05	-1.6438	1.1053	0.17617
0.10	-1.7692	0.95375	0.32974
0.15	-1.8947	0.80210	0.46069
0.20	-2.0201	0.65038	0.56903
0.25	-2.1455	0.49860	0.65473
0.30	-2.2709	0.34675	0.71778
0.35	-2.3963	0.19484	0.75817
0.40	-2.5215	0.042872	0.77589
0.45	-2.6468	-0.10913	0.77095
0.50	-2.7720	-0.26117	0.74333
0.55	-2.8972	-0.41322	0.69303
0.60	-3.0223	-0.56528	0.62005
0.65	-3.1474	-0.71731	0.52440
0.70	-3.2725	-0.86929	0.40607
0.75	-3.3975	-1.0212	0.26508
0.80	-3.5226	-1.1730	0.10142
0.85	-3.6478	-1.3248	-0.084882
0.86	-3.6728	-1.3551	-0.12484
0.86	-3.6728	-1.3551	-0.12486
0.90	-3.7729	-1.4763	-0.29378
0.95	-3.8982	-1.6277	-0.52528
1.00	-4.0236	-1.7788	-0.77935

LC5: Load case: 5: Element 5:

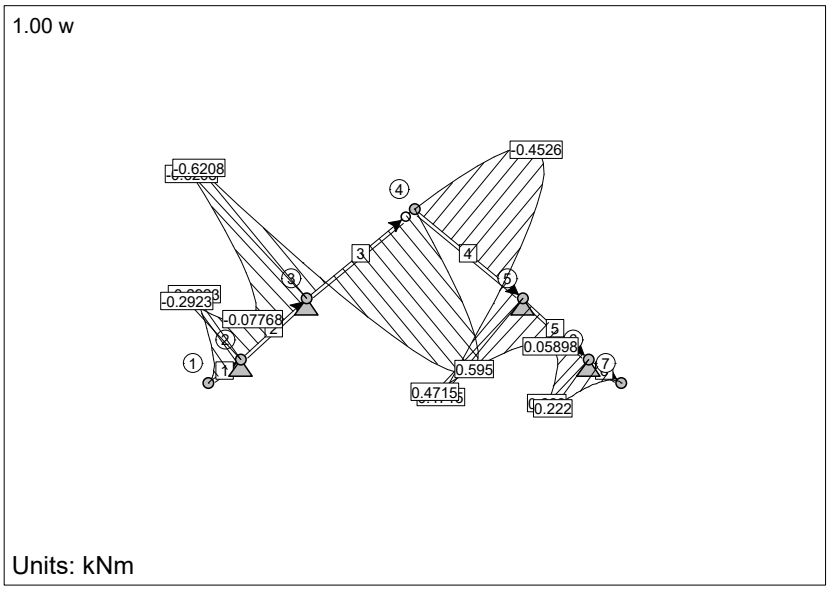
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.81862	1.0662	-0.77934
0.05	0.73731	0.97835	-0.68170
0.10	0.65597	0.89055	-0.59243
0.15	0.57461	0.80276	-0.51156
0.20	0.49322	0.71500	-0.43907
0.25	0.41182	0.62724	-0.37496
0.30	0.33041	0.53950	-0.31923
0.35	0.24898	0.45177	-0.27188
0.40	0.16755	0.36405	-0.23292
0.45	0.086106	0.27633	-0.20233
0.50	0.0046592	0.18861	-0.18013
0.55	-0.076792	0.10090	-0.16630
0.60	-0.15825	0.013197	-0.16085
0.65	-0.23971	-0.074507	-0.16378
0.70	-0.32117	-0.16221	-0.17508
0.75	-0.40264	-0.24990	-0.19477
0.80	-0.48411	-0.33759	-0.22282
0.85	-0.56559	-0.42528	-0.25926
0.90	-0.64707	-0.51295	-0.30407
0.95	-0.72857	-0.60061	-0.35726
1.00	-0.81008	-0.68827	-0.41882

LC5: Load case: 5: Element 6:

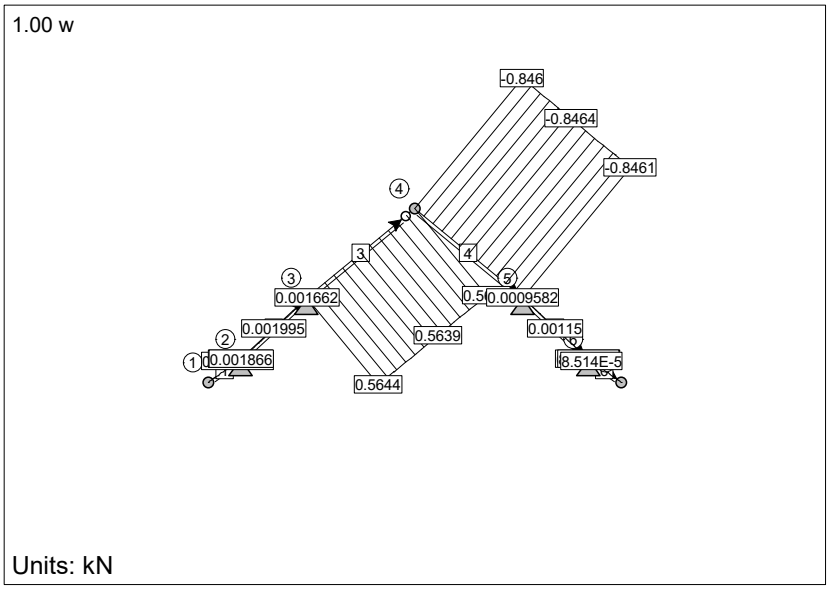
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.69607	0.97380	-0.41882
0.05	0.66129	0.92510	-0.37798
0.10	0.62650	0.87640	-0.33924
0.15	0.59171	0.82770	-0.30259
0.20	0.55692	0.77900	-0.26804
0.25	0.52212	0.73030	-0.23558
0.30	0.48732	0.68161	-0.20522
0.35	0.45252	0.63292	-0.17695
0.40	0.41772	0.58423	-0.15077
0.45	0.38291	0.53554	-0.12669
0.50	0.34811	0.48685	-0.10470
0.55	0.31330	0.43816	-0.084807
0.60	0.27849	0.38948	-0.067008
0.65	0.24368	0.34079	-0.051303
0.70	0.20887	0.29211	-0.037692
0.75	0.17406	0.24342	-0.026175
0.80	0.13925	0.19474	-0.016752
0.85	0.10443	0.14605	-0.0094230
0.90	0.069623	0.097369	-0.0041880
0.95	0.034812	0.048684	-0.0010470
1.00	0.00000	0.00000	0.00000

AMSES Frame2D -
Registred to: Statika Fonda

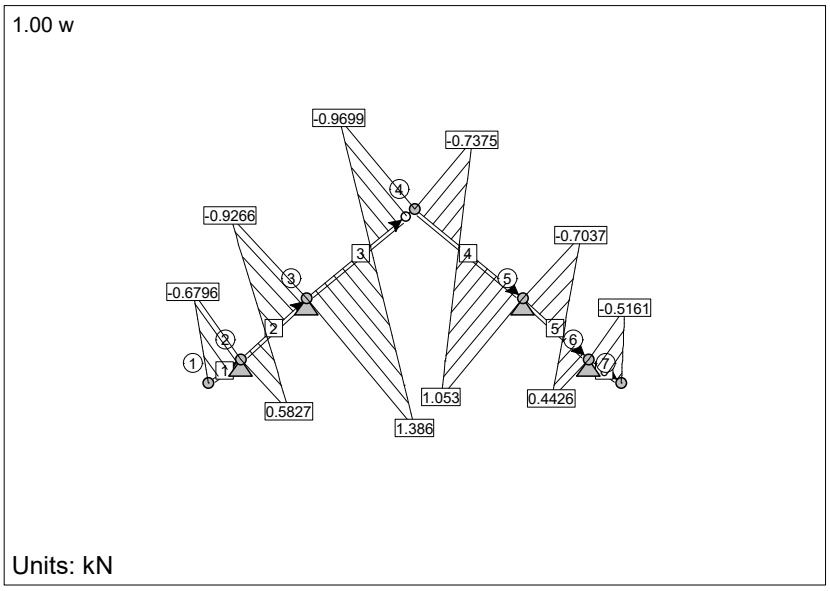
LC6: Load case: 6: Bending Moments My



LC6: Load case: 6: Axial Forces Fx



LC6: Load case: 6: Shear Forces Fz



AMSES Frame2D -
Registred to: Statika Fonda

LC6: Load case: 6: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.014808	0.020725	0.018603
2:	-	-	0.012054
3:	-	-	-0.020227
4:	0.0012311	0.00034259	0.033478

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	-0.015360
6:	-	-	0.0091539
7:	0.011242	-0.015742	0.014128

LC6: Load case: 6: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	-0.79295	-0.97864	-
3:	-1.9469	-1.3901	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-1.8029	0.78768	-
6:	-0.60031	0.74510	-

LC6: Load case: 6: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	1.1032E-05	-0.033979	-0.00073075
0.10	2.2057E-05	-0.067958	-0.0029230
0.15	3.3058E-05	-0.10194	-0.0065767
0.20	4.4005E-05	-0.13592	-0.011692
0.25	5.4859E-05	-0.16990	-0.018269
0.30	6.5565E-05	-0.20388	-0.026307
0.35	7.6061E-05	-0.23785	-0.035807
0.40	8.6271E-05	-0.27183	-0.046768
0.45	9.6107E-05	-0.30581	-0.059191
0.50	0.00010547	-0.33979	-0.073075

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.00011425	-0.37377	-0.088421
0.60	0.00012232	-0.40775	-0.10523
0.65	0.00012956	-0.44173	-0.12350
0.70	0.00013580	-0.47571	-0.14323
0.75	0.00014091	-0.50969	-0.16442
0.80	0.00014470	-0.54367	-0.18707
0.85	0.00014701	-0.57765	-0.21119
0.90	0.00014762	-0.61163	-0.23676
0.95	0.00014635	-0.64560	-0.26380
1.00	0.00014298	-0.67958	-0.29230

LC6: Load case: 6: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.0018663	0.58270	-0.29230
0.05	0.0018997	0.50723	-0.24024
0.10	0.0019252	0.43177	-0.19539
0.15	0.0019445	0.35630	-0.15775
0.20	0.0019591	0.28084	-0.12732
0.25	0.0019702	0.20537	-0.10410
0.30	0.0019787	0.12991	-0.088086
0.35	0.0019852	0.054443	-0.079281
0.40	0.0019901	-0.021022	-0.077685
0.45	0.0019935	-0.096486	-0.083297
0.50	0.0019950	-0.17195	-0.096118

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.0019943	-0.24742	-0.11615
0.60	0.0019905	-0.32288	-0.14339
0.65	0.0019826	-0.39834	-0.17783
0.70	0.0019692	-0.47381	-0.21949
0.75	0.0019487	-0.54927	-0.26836
0.80	0.0019191	-0.62474	-0.32443
0.85	0.0018782	-0.70020	-0.38771
0.90	0.0018236	-0.77567	-0.45820
0.95	0.0017525	-0.85113	-0.53590
1.00	0.0016618	-0.92660	-0.62081

AMSES Frame2D -
Registred to: Statika Fonda

LC6: Load case: 6: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.56443	1.3863	-0.62081
0.05	0.56452	1.2684	-0.42282
0.10	0.56454	1.1505	-0.24241
0.14	0.56452	1.0562	-0.11076
0.14	0.56452	1.0562	-0.11073
0.15	0.56451	1.0327	-0.079584
0.20	0.56445	0.91482	0.065664
0.25	0.56436	0.79700	0.19333
0.30	0.56426	0.67919	0.30343
0.35	0.56417	0.56139	0.39596
0.40	0.56408	0.44360	0.47091
0.45	0.56401	0.32581	0.52829
0.50	0.56396	0.20804	0.56811
0.55	0.56394	0.090264	0.59036
0.60	0.56394	-0.027508	0.59504
0.65	0.56397	-0.14528	0.58215
0.70	0.56403	-0.26305	0.55170
0.75	0.56412	-0.38083	0.50367
0.80	0.56422	-0.49862	0.43808
0.85	0.56434	-0.61641	0.35492
0.90	0.56446	-0.73422	0.25419
0.95	0.56458	-0.85204	0.13588
1.00	0.56468	-0.96987	0.00000

LC6: Load case: 6: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.84598	-0.73745	0.00000
0.05	-0.84603	-0.64794	-0.10333
0.10	-0.84610	-0.55842	-0.19330
0.15	-0.84617	-0.46888	-0.26992
0.20	-0.84624	-0.37933	-0.33318
0.25	-0.84630	-0.28977	-0.38308
0.30	-0.84635	-0.20020	-0.41962
0.35	-0.84638	-0.11063	-0.44280
0.40	-0.84640	-0.021052	-0.45263
0.45	-0.84641	0.068525	-0.44909
0.50	-0.84639	0.15810	-0.43218
0.55	-0.84636	0.24767	-0.40192
0.60	-0.84632	0.33723	-0.35830
0.65	-0.84627	0.42679	-0.30131
0.70	-0.84622	0.51633	-0.23097
0.75	-0.84616	0.60587	-0.14728
0.80	-0.84611	0.69538	-0.050228
0.85	-0.84607	0.78488	0.060173
0.86	-0.84607	0.80277	0.083843
0.86	-0.84607	0.80279	0.083867
0.90	-0.84606	0.87436	0.18392
0.95	-0.84607	0.96381	0.32102
1.00	-0.84612	1.0532	0.47145

LC6: Load case: 6: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00095817	-0.70372	0.47145
0.05	0.0010105	-0.64640	0.40697
0.10	0.0010515	-0.58909	0.34796
0.15	0.0010830	-0.53177	0.29442
0.20	0.0011066	-0.47446	0.24636
0.25	0.0011236	-0.41714	0.20378
0.30	0.0011355	-0.35983	0.16667
0.35	0.0011432	-0.30251	0.13503
0.40	0.0011478	-0.24520	0.10887
0.45	0.0011500	-0.18788	0.088186
0.50	0.0011504	-0.13057	0.072976
0.55	0.0011495	-0.073255	0.063241
0.60	0.0011475	-0.015940	0.058981
0.65	0.0011447	0.041375	0.060196
0.70	0.0011409	0.098690	0.066886
0.75	0.0011360	0.15601	0.079051
0.80	0.0011296	0.21332	0.096690
0.85	0.0011212	0.27064	0.11981
0.90	0.0011101	0.32795	0.14840
0.95	0.0010954	0.38526	0.18246
1.00	0.0010761	0.44258	0.22200

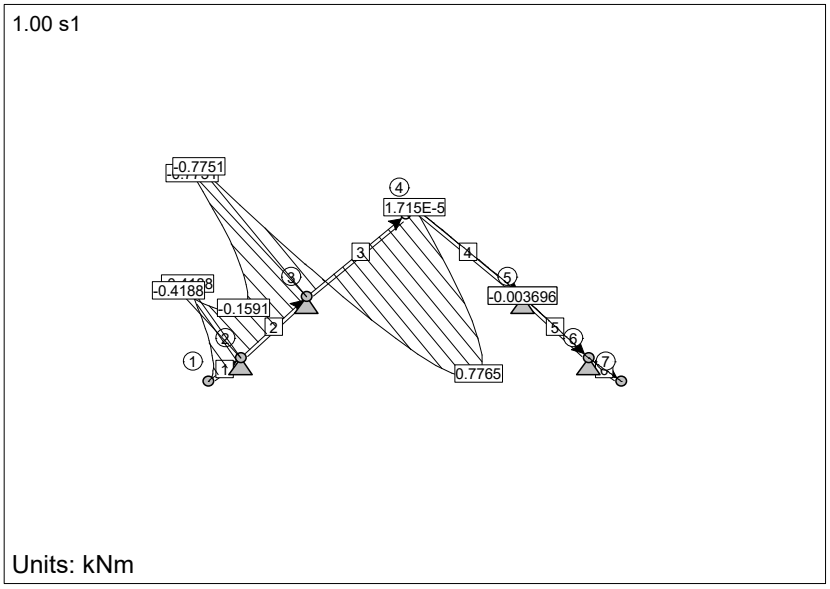
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LC6: Load case: 6: Element 6:

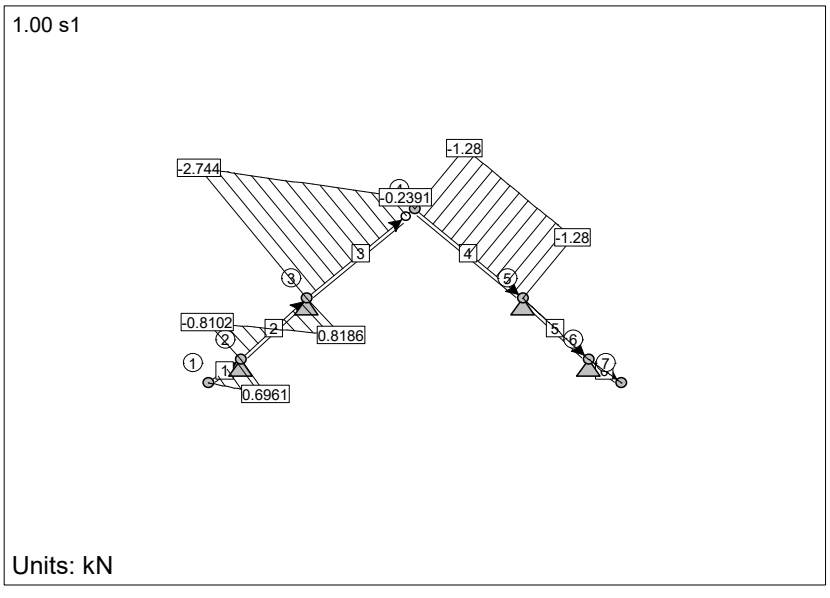
Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	8.2462E-05	-0.51614	0.22200
0.05	8.4409E-05	-0.49033	0.20035
0.10	8.5143E-05	-0.46453	0.17982
0.15	8.4788E-05	-0.43872	0.16039
0.20	8.3461E-05	-0.41291	0.14208
0.25	8.1273E-05	-0.38710	0.12487
0.30	7.8328E-05	-0.36130	0.10878
0.35	7.4725E-05	-0.33549	0.093795
0.40	7.0553E-05	-0.30968	0.079920
0.45	6.5896E-05	-0.28388	0.067155
0.50	6.0832E-05	-0.25807	0.055500

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	5.5432E-05	-0.23226	0.044955
0.60	4.9759E-05	-0.20646	0.035520
0.65	4.3871E-05	-0.18065	0.027195
0.70	3.7817E-05	-0.15484	0.019980
0.75	3.1641E-05	-0.12903	0.013875
0.80	2.5381E-05	-0.10323	0.0088800
0.85	1.9067E-05	-0.077421	0.0049950
0.90	1.2722E-05	-0.051614	0.0022200
0.95	0.00000	-0.025807	0.00055500
1.00	0.00000	0.00000	0.00000

LC7: Load case: 7: Bending Moments My

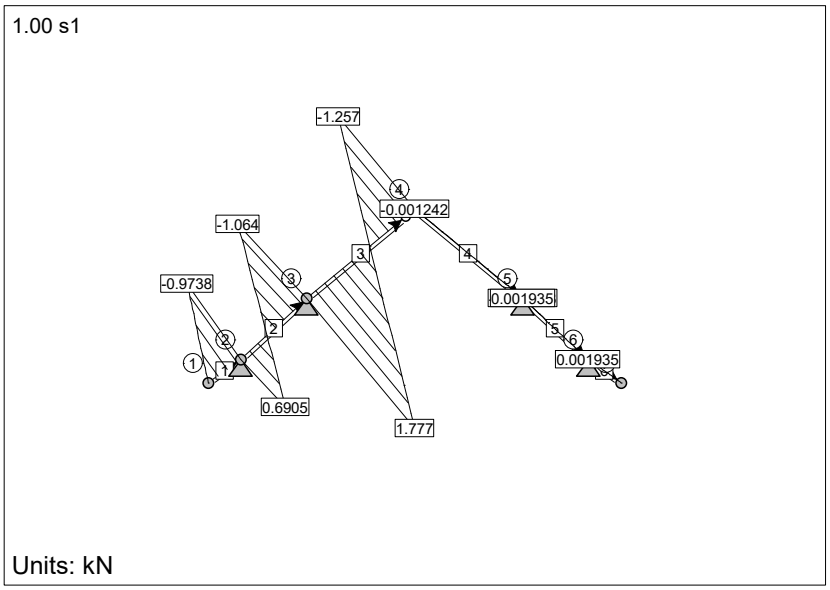


LC7: Load case: 7: Axial Forces Fx



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LC7: Load case: 7: Shear Forces Fz



LC7: Load case: 7: Nodal Displacements

Node	ux [cm]	uz [cm]	fiy [deg]
1:	0.022859	0.032222	0.028659
2:	-	-	0.019276
3:	-	-	-0.028137
4:	-0.00022549	0.0029973	0.00061523

Node	ux [cm]	uz [cm]	fiy [deg]
5:	-	-	0.00018391
6:	-	-	-9.1955E-05
7:	-8.0246E-05	0.00011234	-9.1955E-05

LC7: Load case: 7: Reactions

Node	Fx [kN]	Fz [kN]	My [kNm]
2:	0.12347	-2.2543	-
3:	0.86220	-4.4538	-

Node	Fx [kN]	Fz [kN]	My [kNm]
5:	-0.98435	-0.81726	-
6:	-0.0013163	0.0014179	-

LC7: Load case: 7: Element 1:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.034811	-0.048685	-0.0010470
0.10	0.069623	-0.097369	-0.0041880
0.15	0.10443	-0.14605	-0.0094230
0.20	0.13925	-0.19474	-0.016752
0.25	0.17406	-0.24342	-0.026175
0.30	0.20887	-0.29211	-0.037692
0.35	0.24368	-0.34079	-0.051303
0.40	0.27849	-0.38948	-0.067008
0.45	0.31330	-0.43817	-0.084808
0.50	0.34810	-0.48685	-0.10470

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.38291	-0.53554	-0.12669
0.60	0.41772	-0.58423	-0.15077
0.65	0.45252	-0.63292	-0.17695
0.70	0.48732	-0.68161	-0.20522
0.75	0.52212	-0.73030	-0.23558
0.80	0.55692	-0.77900	-0.26804
0.85	0.59171	-0.82770	-0.30259
0.90	0.62650	-0.87640	-0.33924
0.95	0.66129	-0.92510	-0.37798
1.00	0.69607	-0.97380	-0.41882

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LC7: Load case: 7: Element 2:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-0.81015	0.69050	-0.41882
0.05	-0.72864	0.60285	-0.35705
0.10	-0.64715	0.51519	-0.30365
0.15	-0.56566	0.42751	-0.25862
0.20	-0.48418	0.33983	-0.22197
0.25	-0.40271	0.25214	-0.19370
0.30	-0.32124	0.16445	-0.17380
0.35	-0.23978	0.076745	-0.16228
0.40	-0.15832	-0.010958	-0.15914
0.45	-0.076866	-0.098666	-0.16437
0.50	0.0045851	-0.18638	-0.17799

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.086032	-0.27409	-0.19998
0.60	0.16747	-0.36181	-0.23035
0.65	0.24891	-0.44953	-0.26910
0.70	0.33033	-0.53727	-0.31624
0.75	0.41175	-0.62501	-0.37175
0.80	0.49315	-0.71276	-0.43564
0.85	0.57453	-0.80052	-0.50792
0.90	0.65590	-0.88831	-0.58859
0.95	0.73724	-0.97611	-0.67763
1.00	0.81856	-1.0639	-0.77507

LC7: Load case: 7: Element 3:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-2.7443	1.7768	-0.77507
0.05	-2.6189	1.6255	-0.52132
0.10	-2.4936	1.4740	-0.29016
0.14	-2.3935	1.3527	-0.12152
0.14	-2.3935	1.3527	-0.12150
0.15	-2.3685	1.3224	-0.081611
0.20	-2.2433	1.1707	0.10434
0.25	-2.1182	1.0189	0.26765
0.30	-1.9932	0.86706	0.40830
0.35	-1.8681	0.71519	0.52630
0.40	-1.7430	0.56331	0.62165
0.45	-1.6179	0.41143	0.69435

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.50	-1.4927	0.25957	0.74439
0.55	-1.3675	0.10773	0.77179
0.60	-1.2423	-0.044077	0.77654
0.65	-1.1170	-0.19584	0.75865
0.70	-0.99161	-0.34756	0.71812
0.75	-0.86622	-0.49923	0.65496
0.80	-0.74080	-0.65085	0.56918
0.85	-0.61536	-0.80243	0.46079
0.90	-0.48992	-0.95398	0.32979
0.95	-0.36447	-1.1055	0.17620
1.00	-0.23906	-1.2570	1.7146E-05

LC7: Load case: 7: Element 4:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	-1.2795	-0.0012422	0.00000
0.05	-1.2795	-0.0012422	-0.00018529
0.10	-1.2795	-0.0012421	-0.00037057
0.15	-1.2795	-0.0012420	-0.00055584
0.20	-1.2795	-0.0012418	-0.00074108
0.25	-1.2795	-0.0012416	-0.00092630
0.30	-1.2795	-0.0012413	-0.0011115
0.35	-1.2795	-0.0012410	-0.0012966
0.40	-1.2795	-0.0012406	-0.0014817
0.45	-1.2795	-0.0012402	-0.0016667
0.50	-1.2795	-0.0012398	-0.0018517
0.55	-1.2795	-0.0012393	-0.0020366

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.60	-1.2795	-0.0012387	-0.0022214
0.65	-1.2795	-0.0012381	-0.0024061
0.70	-1.2795	-0.0012375	-0.0025908
0.75	-1.2795	-0.0012368	-0.0027753
0.80	-1.2795	-0.0012360	-0.0029597
0.85	-1.2795	-0.0012352	-0.0031440
0.86	-1.2795	-0.0012351	-0.0031809
0.86	-1.2795	-0.0012351	-0.0031809
0.90	-1.2795	-0.0012344	-0.0033282
0.95	-1.2795	-0.0012335	-0.0035123
1.00	-1.2795	-0.0012326	-0.0036962

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LC7: Load case: 7: Element 5:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.0019347	-0.0036962
0.05	0.00000	0.0019347	-0.0035114
0.10	0.00000	0.0019347	-0.0033266
0.15	0.00000	0.0019347	-0.0031418
0.20	0.00000	0.0019347	-0.0029570
0.25	0.00000	0.0019347	-0.0027722
0.30	0.00000	0.0019347	-0.0025873
0.35	0.00000	0.0019347	-0.0024025
0.40	0.00000	0.0019347	-0.0022177
0.45	0.00000	0.0019347	-0.0020329
0.50	0.00000	0.0019347	-0.0018481

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.00000	0.0019347	-0.0016633
0.60	0.00000	0.0019347	-0.0014785
0.65	0.00000	0.0019347	-0.0012937
0.70	0.00000	0.0019347	-0.0011089
0.75	0.00000	0.0019347	-0.00092405
0.80	0.00000	0.0019347	-0.00073924
0.85	0.00000	0.0019347	-0.00055443
0.90	0.00000	0.0019347	-0.00036962
0.95	0.00000	0.0019347	-0.00018481
1.00	0.00000	0.0019347	0.00000

LC7: Load case: 7: Element 6:

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.00000	0.00000	0.00000
0.05	0.00000	0.00000	0.00000
0.10	0.00000	0.00000	0.00000
0.15	0.00000	0.00000	0.00000
0.20	0.00000	0.00000	0.00000
0.25	0.00000	0.00000	0.00000
0.30	0.00000	0.00000	0.00000
0.35	0.00000	0.00000	0.00000
0.40	0.00000	0.00000	0.00000
0.45	0.00000	0.00000	0.00000
0.50	0.00000	0.00000	0.00000

Pos	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.00000	0.00000	0.00000
0.60	0.00000	0.00000	0.00000
0.65	0.00000	0.00000	0.00000
0.70	0.00000	0.00000	0.00000
0.75	0.00000	0.00000	0.00000
0.80	0.00000	0.00000	0.00000
0.85	0.00000	0.00000	0.00000
0.90	0.00000	0.00000	0.00000
0.95	0.00000	0.00000	0.00000
1.00	0.00000	0.00000	0.00000

Element 3 design results (Building code - EC5):

Cross-section [cm]
[10/20], h = 20.00, b = 10.00

Element supports width left = 0.00 cm, right = 0.00 cm

Timber: C24 (solid)

Supports and buckling fields around y axis

Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

Supports and buckling fields around z axis

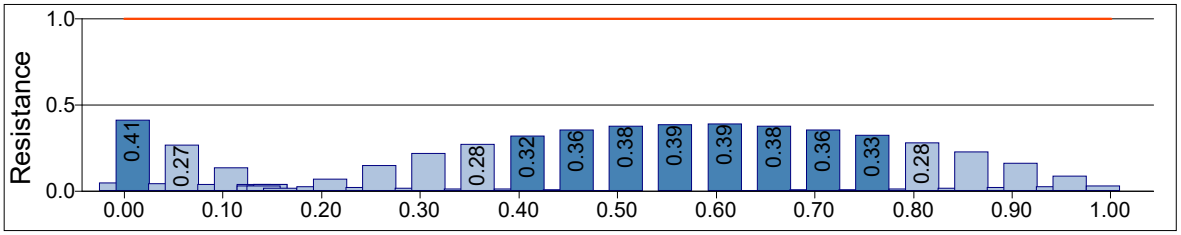
Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

Element service class 2

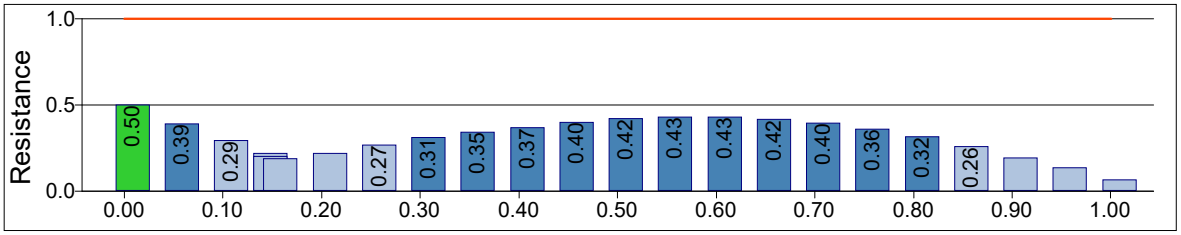
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Ultimate Limit States

Shear & CS Resistance

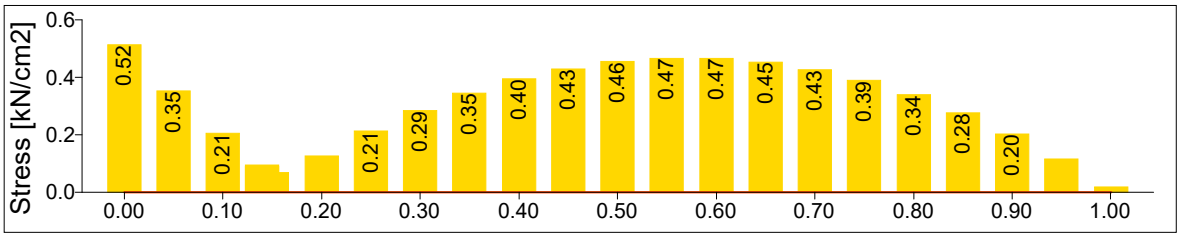


Stability Resistance

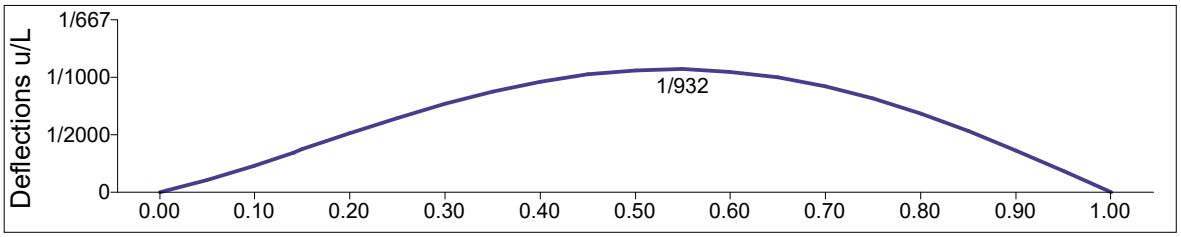


Serviceability Limit States

Serviceability stresses



Element deflections (local)



Element 5 design results (Building code - EC5):

Cross-section [cm]
[10/20], h = 20.00, b = 10.00

Element supports width left = 0.00 cm, right = 0.00 cm

Timber: C24 (solid)

Supports and buckling fields around y axis

Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

Supports and buckling fields around z axis

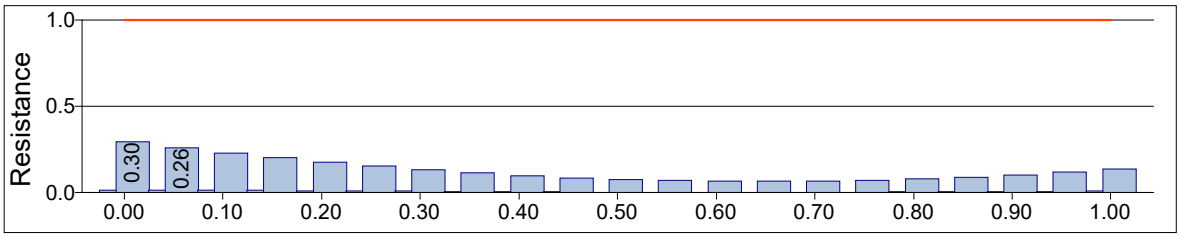
Field	x start	x end	buckl. coeff.
1	0.000	1.000	1.000

Element service class 2

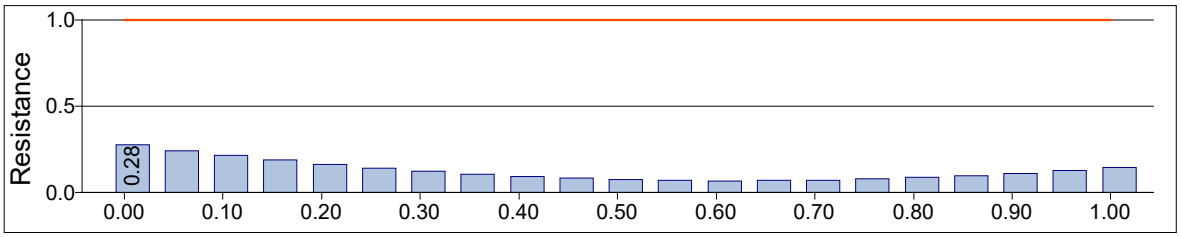
Ultimate Limit States

AMSES Frame2D -
Registred to: Statika Fonda

Shear & CS Resistance

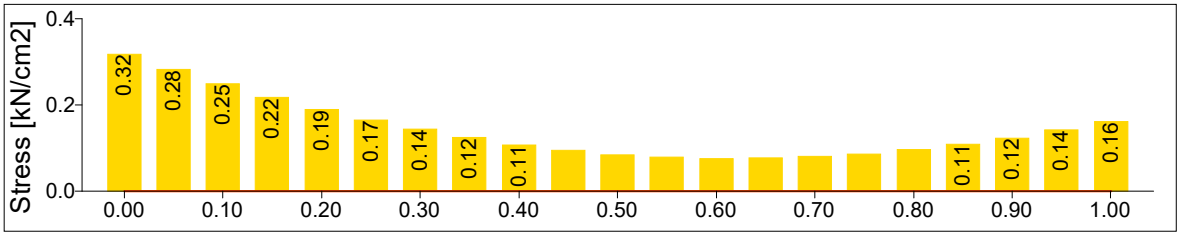


Stability Resistance

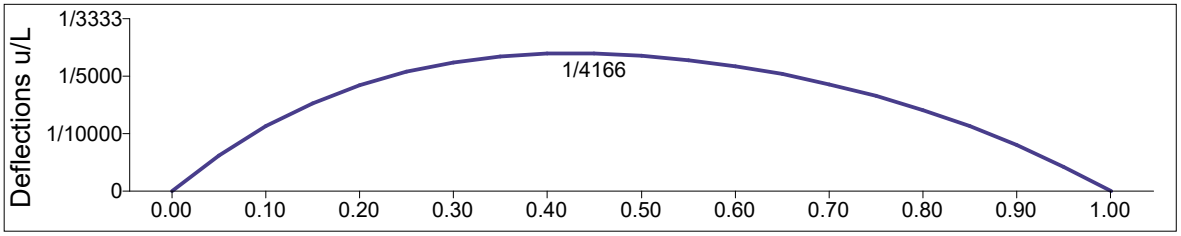


Serviceability Limit States

Serviceability stresses



Element deflections (local)



AMSES Frame2D -
Registred to: Statika Fonda